

TECHNICAL MEMORANDUM No. 1

Crown Isle Villas Mini Home Development Pre-Design Servicing Report

Silverado Land Corporation
399 Clubhouse Drive,
Courtenay, BC V9N 2N3
File 9109-193-TM1

Issued: June 25th, 2021

Reference Drawings: 9109-193-P1, P2

Previous Issue Date: April 9, 2021

Figures 1 - 3.

1. Objective

This Pre-Design Servicing Report is provided in support of the proposed 28-unit mini home development on **Lot 2, Plan VIP78417 and Rem Lot A Plan VIP78730**, located off Clubhouse Drive, east of the Crown Isle Clubhouse and Villas. Included with this submission are drawings 9109-193-P1, showing the proposed layout and 9109-193-P2, showing the site servicing concepts, and Figures 1 through 3 providing additional details.

2. Background

This development, bounded by the golf course first hole and Ryan Road, was partially pre-serviced in 2004, when it was planned to be an extension of the Crown Isle Villa development. The sanitary sewer was installed to the Majestic Pump Station and stubs were left for the future villa buildings. The storm sewer, watermain and hydro and telephone conduit were extended approximately 100 m past the eastern-most constructed villa building. Drawing 9109-193-P2 shows the existing sewer and water utilities that were pre-installed. The proposed development will utilize a portion of the existing storm and water utilities, and new services will be installed to complete this project.

3. Site Grading (See Drawing 9109-P2)

The site slopes from west to east at about 4.5%. It is anticipated that the proposed units will be constructed at a slightly higher elevation than the access road. The access road will slope down to the terminus near unit number 15. It is anticipated that the eastern portion of the site, including units 8 to 19 will require structural fill to raise the ground level and enable a gravity sanitary service.

We understand that Silverado Land Corporation will have a geotechnical analysis prepared for this development to assess the subsurface geology, define the depths to suitable subgrade and identify areas where groundwater infiltration may be possible.

4. Road Works (See Drawing 9109-193-P1)

The 6.0m wide access road and parking areas will be gravel surfaced.

The hammerhead style turnaround at the end of the road conforms to the City's drawing CSSD R4. Details of the proposed road access and a typical section are shown on drawing 9109-193-P1.

5. Storm Drainage (See Figure 1)

An existing 250mm diameter pipe runs through the development, conveying drainage from the existing 1.2 ha Crown Isle Villa development, discharging to the ditch on the south side of Ryan Road. This pipe will be intercepted and re-routed around the north side of the proposed 8-plex, discharging into the existing ditch near the current outlet location. New structures within the proposed mini-home development will not connect to the existing pipe. A new storm sewer will be constructed from the west side of the site to the east side where it will connect to a new oil/grit separator before being routed to the existing infiltration area located on the golf course. An easement between the upstream property and the golf course property is anticipated to include the new piping draining to the infiltration area.

We understand that the existing infiltration drainwell structures located on the golf course near the east side of this proposed development have been successfully infiltrating runoff from the existing 9.3 ha catchment area over the last 30 years. The infiltration characteristics of this area will be reviewed to better understand the capacity of the subsoils to infiltrate.

It is anticipated that each building will have a separate storm sewer connection for foundation and roof drainage systems. We understand that the proposed 8-plex will have a common foundation, foundation drains and roof drainage system, so it is anticipated to have one storm connection for the 8-unit building.

Impacts on Downstream Infrastructure

The Impacts to the downstream drainage infrastructure will be reviewed during the investigation and design stage and refined with further storm water modelling. Although there is no direct connection between the infiltration area and the Ryan Road ditch, the topography suggests that if the depressed area around the infiltration drainwells overtopped, the drainage would flow towards Ryan Road. We understand that this infiltration area has never overtopped in the last 30 years. The Ryan Road ditch terminates at an existing 300 CSP culvert that crosses to the north side of the Ryan Road.

This culvert, presumably installed when Ryan Road was constructed, shows very little evidence that it conveys any drainage. There is no defined ditch at the outlet and little evidence of erosion or scour, even though the culvert's 7.6 ha catchment area includes the impervious surfaces of Ryan Road and the existing Villa development. This is further evidence that much of the runoff

from this catchment does not end up at this culvert and probably infiltrates into the ground before it reaches the inlet.

6. Sanitary Sewers (See Figure 2)

All of the buildings will be serviced by a new gravity sanitary sewer that will be connected to an existing private manhole and trunk sewer on the golf course that services the existing Villa development and conveys sewage to the Majestic Pump Station.

The existing and proposed sanitary sewer servicing concept is shown on Figure 2, and the Sanitary Sewer Analysis – Calculation Sheet is summarized in Table 1. The Table also includes the total population, catchment area and peak flows for the Majestic Pump Station.

For modelling purposes, the additional sanitary loading to the Majestic Pump Station is as follows:

- **City Design Criteria**

From Section 3.2 “Per Capita Flow” of the Bylaw 2929, the design flows applied to this development are as follows:

- Average Dry Weather Flow (ADWF): 360 L/c/d
- Population: 2.4 Capita per MF unit or SF lot

From MMCD Sections 3.4 to 3.6

- $PF = 3.2/P^{0.105}$ (P =population in thousand rounded to nearest thousand).
- Infiltration: 0.06 L/s/ha
- Design Flow $Q=(ADWF \times PF) + \text{Infiltration}$

City Criteria for Commercial Developments:

- Equivalent Residential Units (ERU) = 1 ERU per 1920 sq.ft.
 - Total Proposed Floor Area = 14,784 sq.ft.
 - Total ERU = $14784/1920 = 7.7$ ERU

- **Sanitary Loading from Proposed Development**

Up MH	Dn MH	Area (ha)	ERU	Pop	ADWF (L/day)	PF	Peak Flow (L/s)	I/I (L/s)	Design Flow (L/s)
Majestic PS	n/a	0.8	7.7	18.5	6653	4.9	0.38	0.05	0.43

- **Majestic Pump Station:**

(data from record drawing)

- Type: Duplex Flygt NP 3127 HT 1
Capacity 9.7 lps @ 17.0 m total head. (Single pump)
Peak Flow: 2.6 lps (including the Mini Home Development)

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With an anticipated post development peak flow of 2.6 lps and an estimated current capacity of 9.7 lps, the pump station will be operating below capacity at full build-out for this development.

Each unit will have a separate sanitary sewer service, sized in accordance with the BC Plumbing Code. It is anticipated that the 8-plex will have one sanitary service for the building.

7. Water System (See Figure 3)

The site will be serviced from the existing watermain that runs through the site.

Currently, we understand that each of the different building stratas within the Villa development have individual water meters that are read by the City.

A new master meter and double check valve assembly is anticipated to be installed at the west end of the Mini Home development as shown on drawing 9109-193-P1. Some of the existing watermain will be removed and replaced with a new watermain and a fire hydrant. Each unit will have a water service, sized in accordance with the BC Building code. It is not anticipated that these units will have fire suppression systems.

We understand that the City will run their water model with the proposed water system extension. The proposed water flows are as follows:

- **City Design Criteria**

From Section 2.3 "Per Capita Demand" of the Bylaw 2929, the per capita demands applied to this development are as follows:

- | | |
|---|----------------------------------|
| • Average Daily Demand: | 635 L/c/d |
| • Peak Day (also known as Maximum Day): | 2100 L/c/d |
| • Peak Hour: | 3000 L/c/d |
| • Population: | 2.4 Capita per MF unit or SF lot |

From MMCD Section 2.5 "Fire Flows"

- Fire flows should be determined in accordance with the requirements of the current edition of "Water Supply for Public Fire protection – A Guide to Recommended Practice", published by Fire Underwriters Survey (FUS)
- Single Family Residential: 60 L/s minimum
- Apartments, Townhouses: 90 L/s minimum

FUS calculations have been completed on both a single 14'x40' mini home and the 8 plex. The estimated fire flows for these building are 46 L/s and 105 L/s respectively (see attached FUS calculation forms). The largest fire flow (105 L/s) should be used as the design fire flow.

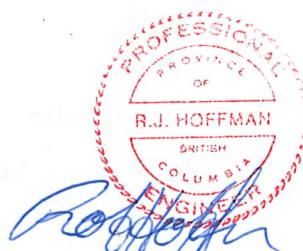
11. Closing

Based on a review of site conditions, an assessment of the development requirements and an evaluation of servicing options, we feel that a viable conceptual design to form the basis of the detailed design work has been presented. We trust that this submission is suitable to support the development permit application for this development. If you have any questions, or require further information to support this application, please contact our office at your earliest convenience.

Yours truly,

KOERS & ASSOCIATES ENGINEERING LTD.

Richard Cave AScT
Project Technologist



Rob Hoffman, P. Eng.
Project Manager

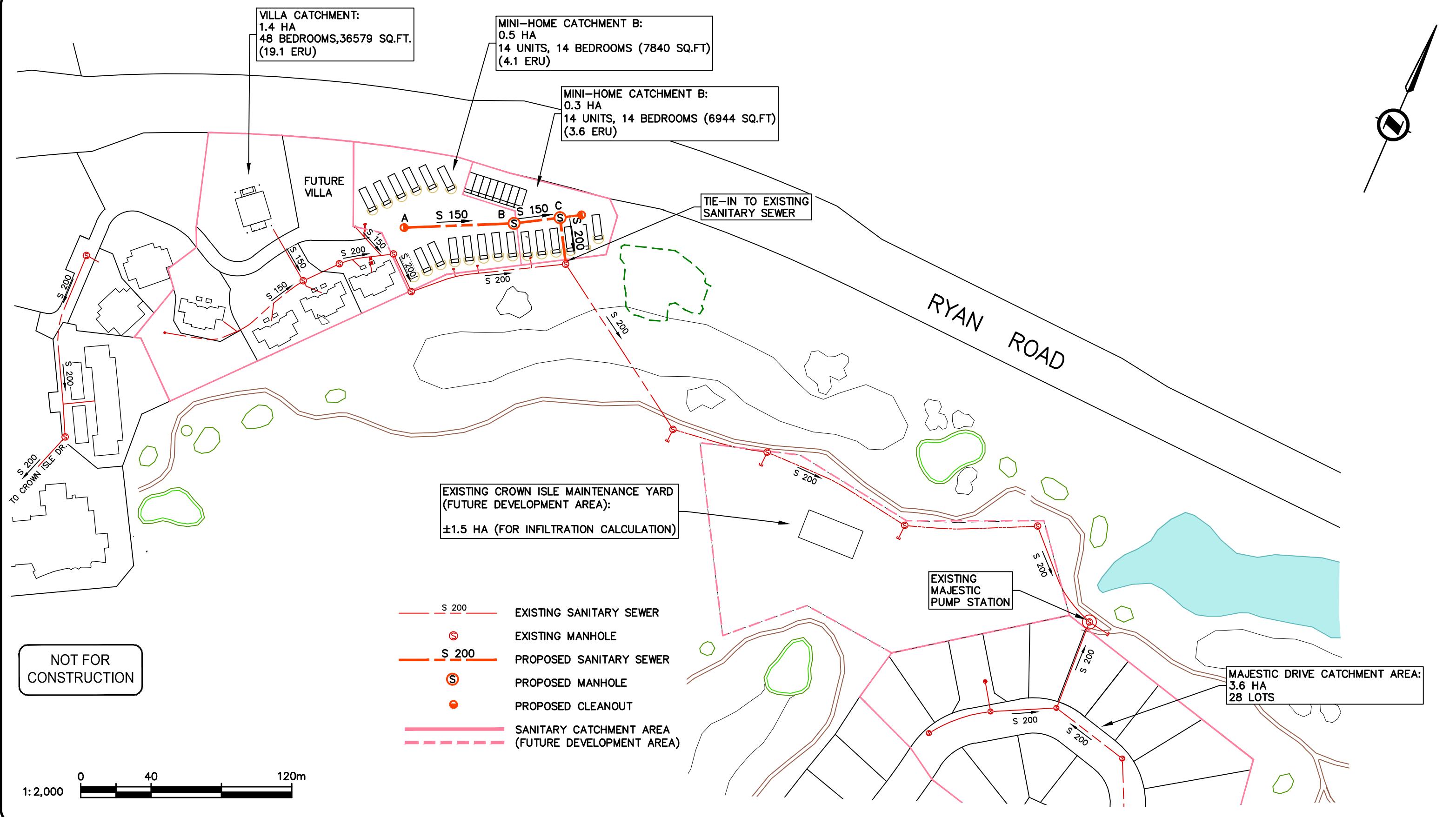
Enclosures: Drawings 9109-193-P1, P2,
Figures 1-3
Table 1
2 x FUS Calculation Forms

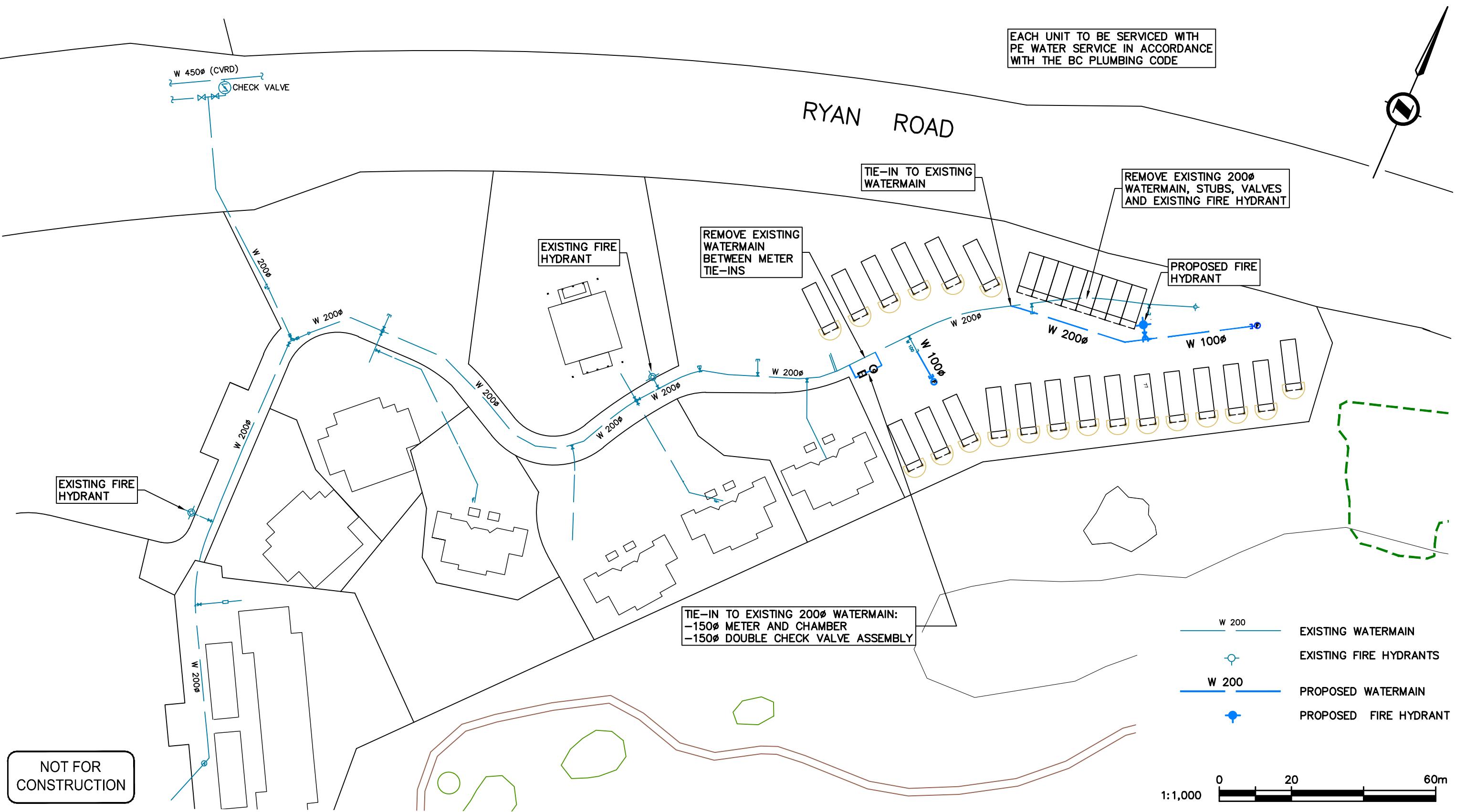
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PRELIMINARY DESIGN WATERMAIN

APPROVED	SCALE	1:1000
DATE	JANUARY 2021	DWG No.
PROJECT No.	9109-193	

FIGURE 2

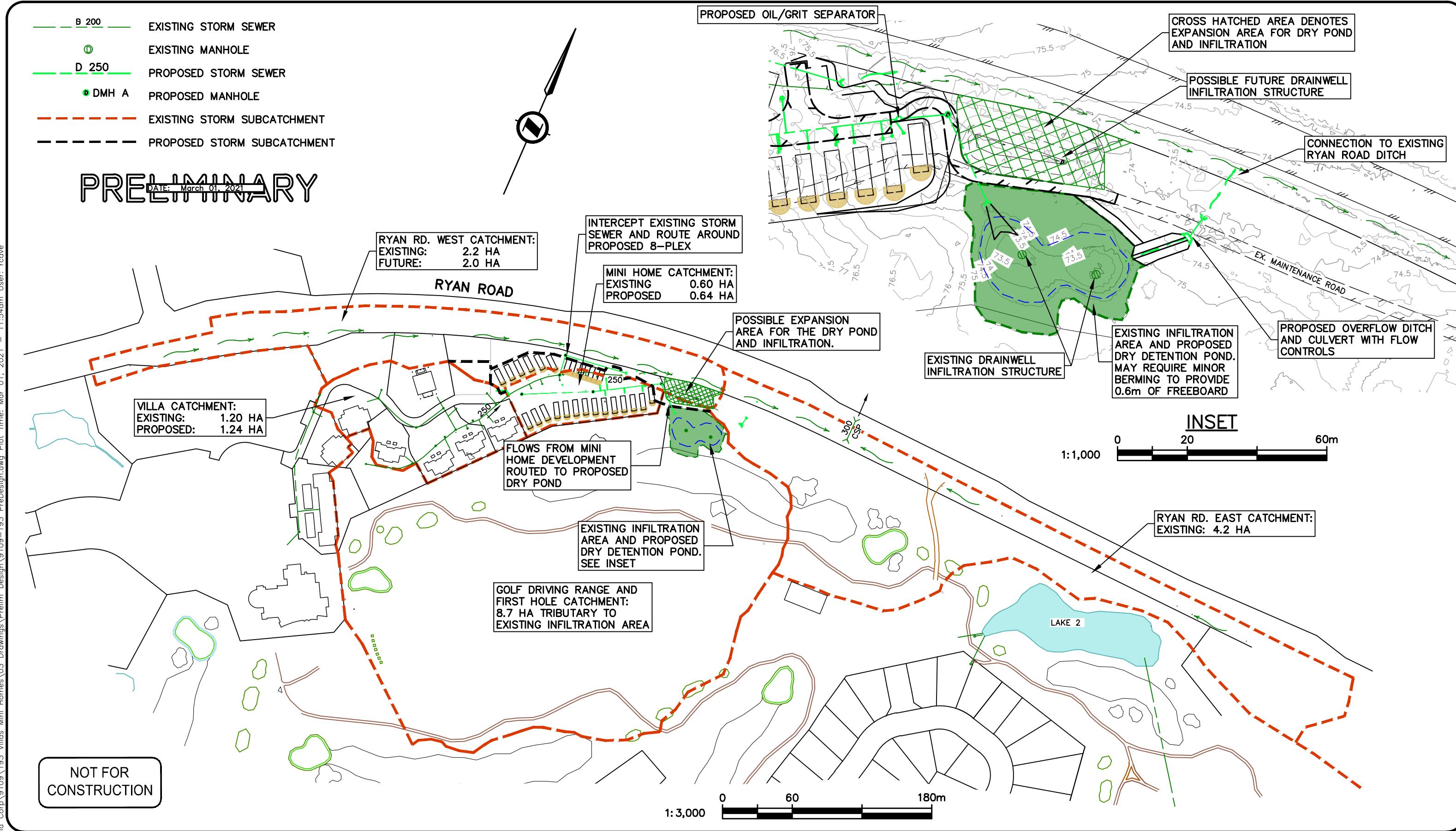
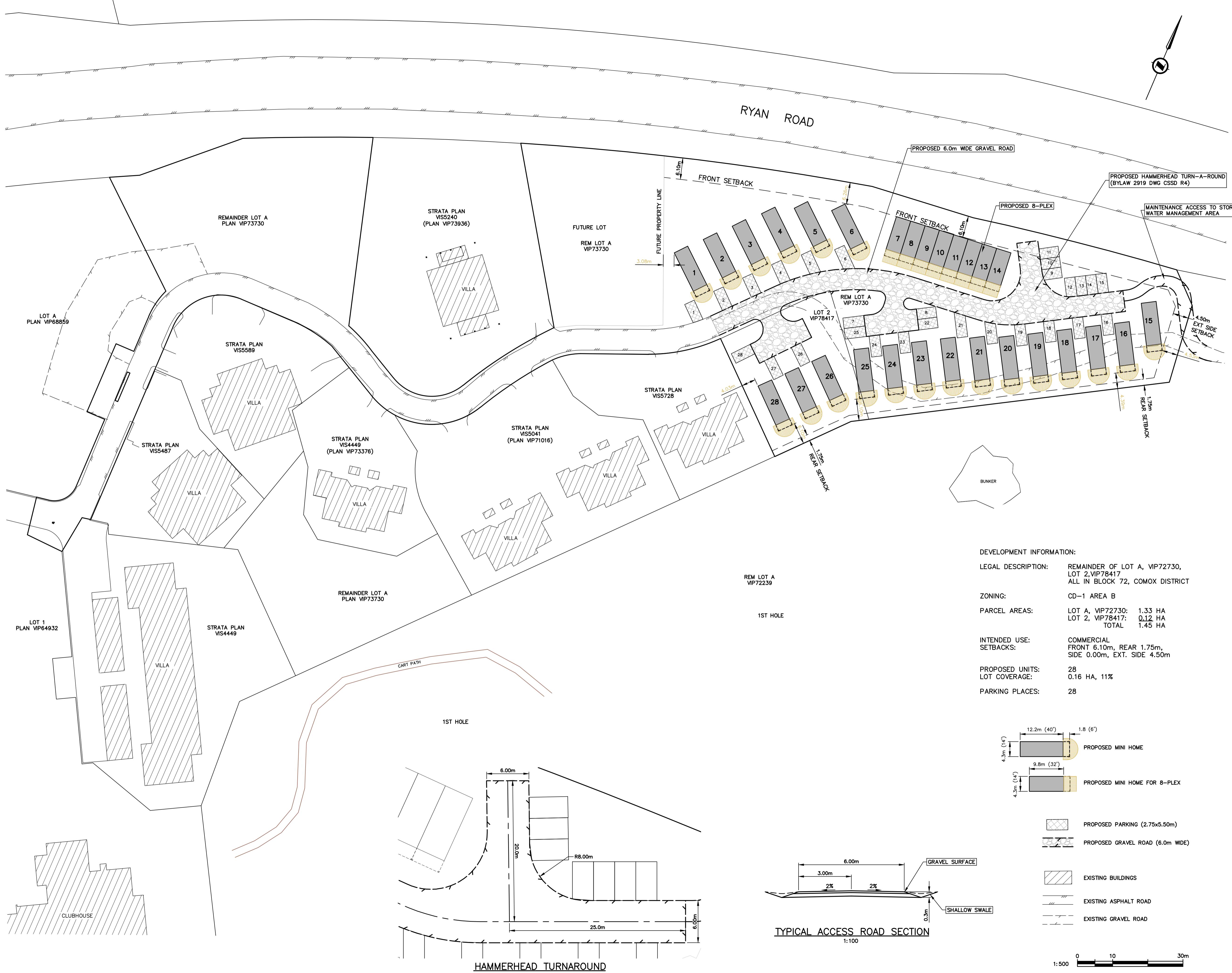
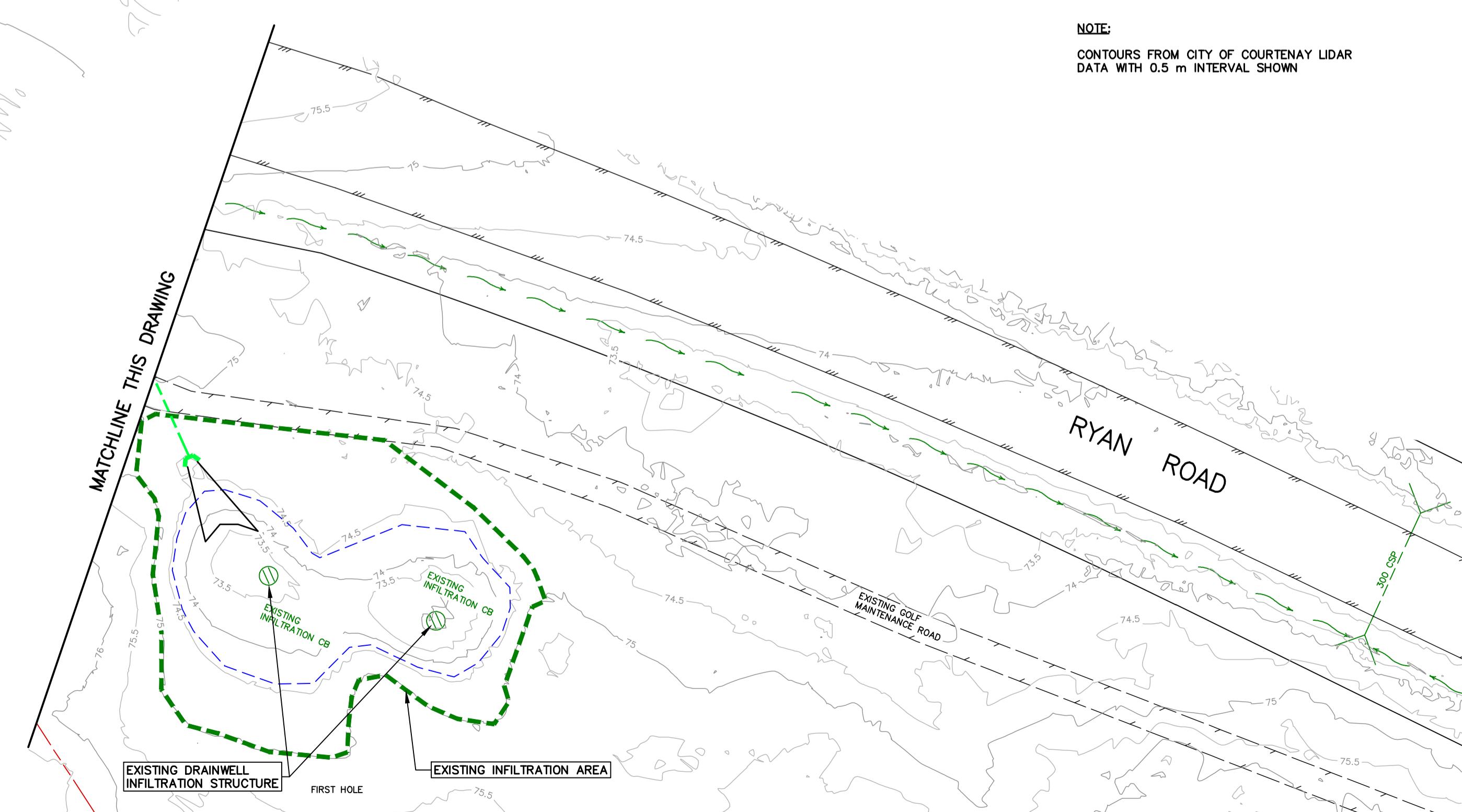


Table 1
SANITARY SEWER FLOW ANALYSIS - CALCULATION SHEET

DP - Servicing Report





CROWN ISLE MINI HOMES

PRELIMINARY SITE SERVICING CONCEPTS