



Our File: 2111-47226-01

# **TECHNICAL MEMO**

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Re	Date
2650 Copperfield Road Traffic Impact Study	May 10, 2022

## 1. Introduction

The purpose of this technical memo is to review the traffic operations for the proposed development located at 2650 Copperfield Road in Courtenay, BC. The following presents our assumptions, analysis results, and recommendations.

# 2. Background Information

### 2.1. Project Description

The proposed development site is located at 2650 Copperfield Road in Courtenay, BC. It is our understanding that Rosebery Investments Ltd, (the Client) plans to develop the existing vacant lot into 22 single-family homes, 2 duplex units, 15 townhomes, and a maximum of 7 carriage homes. Note that the carriage homes will be optional, some of which may never be constructed. Carriage homes are typically residential suites located above detached garages.

The development will have a single site access along Copperfield Road. An overview of the proposed development, including site access, is included in *Attachment A*.

### 2.2. Location

The proposed development is located at 2650 Copperfield Road in Courtenay, BC. The development is located at the west end of Copperfield Road. The study area for the proposed development is shown on *Figure 1*, which includes the following three study intersections:

- Arden Road / Lake Trail Road (2-way stop controlled on Arden Road),
- Arden Road / Copperfield Road (1-way stop controlled on Copperfield Road) and
- Arden Road / Cumberland Road (2-way stop controlled on Arden Road).



### 2.3. Existing Conditions

Existing conditions for roads within the study area are described in the following section. All road classifications are based on the latest *Draft City of Courtenay Official Community Plan* from January 2022.

#### Lake Trail Road

Lake Trail Road is an east-west running two-lane undivided road with a 50 km/h speed limit. The road has bicycle lanes east of Arden Road and paved roadside shoulders west of Arden Road. Lake Trail Road is classified as an arterial road and has a rural cross section with roadside ditches on both sides of the road.

#### Arden Road

Arden Road is a north-south running two-lane undivided road with a 50 km/h speed limit. The road has gravel shoulders on certain portions of the road, as well as a roadside ditch on the west side of the road. The road is fronted by driveways for various single-family homes. Arden Road is classified as a rural collector road.

#### **Cumberland Road**

Cumberland Road is a two-lane undivided road with a 50 km/h speed limit which transitions to 60 km/h further to the west of Arden Road. The road provides a connection to Comox Valley Parkway to the south, and thereby Highway 19, as well as Courtenay's commercial areas to the north. The road cross section includes painted bicycle lanes as well as roadside ditches. Cumberland Road is currently classified as an arterial road.

#### Copperfield Road

Copperfield road is a two-lane undivided local road with a 50 km/h speed limit. The road currently terminates in a dead-end approximately 250m west of Arden Road.

#### 2.4. Future Road Network

In the City of Courtenay Transportation Master Plan (2019), Arden Road / Cumberland Road was identified as a location for potential safety and operational improvements. This may include traffic control upgrades including new signals and / or roundabouts. It is indicated in the report that "in the long-term, it is recommended that the City monitor traffic growth and operations at unsignalized intersections to determine where and when new signals or roundabouts are needed". The Arden Corridor Local Area Plan (2013) further indicates that roundabouts will be considered for intersection treatments along the Arden Road corridor.

Based on the City of Courtenay's existing Official Community Plan *Road Network Map No. 3*, last revised on September 21, 2007, an additional local road connection between Copperfield Road and Cumberland Road was previously considered as shown in *Figure 2*; however, the same road network additions have not been included in more recent documents and OCP updates including the *Draft Official Community Plan* 

from January 2022, the *Arden Corridor Local Area Plan* (2013), or the *City of Courtenay Transportation Master Plan* (2019). As a result, no consideration for additional local road connections to Copperfield Road have been included in this study.

PROPOSED DEVELOPMENT

PREVIOUSLY PROPOSED LOCAL ROADS

AMERICAN ROADS

AMERICA

Figure 2: Future Road Network - City of Courtenay OCP Road Network Map No. 3 (2007) (Modified)

ROAD CLASSIFICATION	EXISTING	FUTURE
ARTERIAL - MAJOR - MINOR		
COLLECTOR - INDUSTRIAL/COMMERCIAL - RESIDENTIAL		
LOCAL - INDUSTRIAL/COMMERCIAL - RESIDENTIAL		
LANES		

## 3. Multimodal Analysis

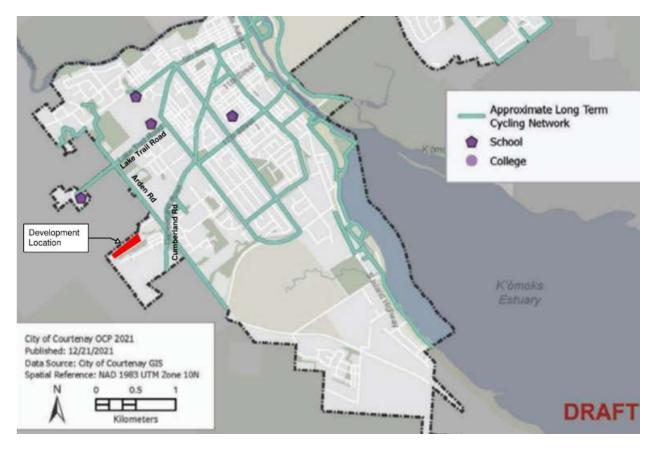
### 3.1. Cycling Network

There are currently several cycling facilities within the vicinity of the development. Painted bike lanes are currently available along Lake Trail Road east of Arden Road as well as along Cumberland Road.

Based on the City of Courtenay's Draft OCP, 2021, additional cycling infrastructure is planned along Arden Road as well as the Lake Trail Road as shown in *Figure 3*. Likewise, in the *Transportation Master Plan* (2019), several cycling improvements in the vicinity of the development have been recommended in a list of medium term (10 year) cycling improvements as shown in *Figure 4*. These improvements include:

- Paved multi-use pathway along Arden Road from Morrison Creek to Comox Valley Parkway
- Paved multi-use pathway along Lake Trail Road from Willemar Avenue to Webdon Road

Figure 3: Long Term Cycling Network Connectivity Opportunities - City of Courtenay OCP Draft, 2022 (Modified)



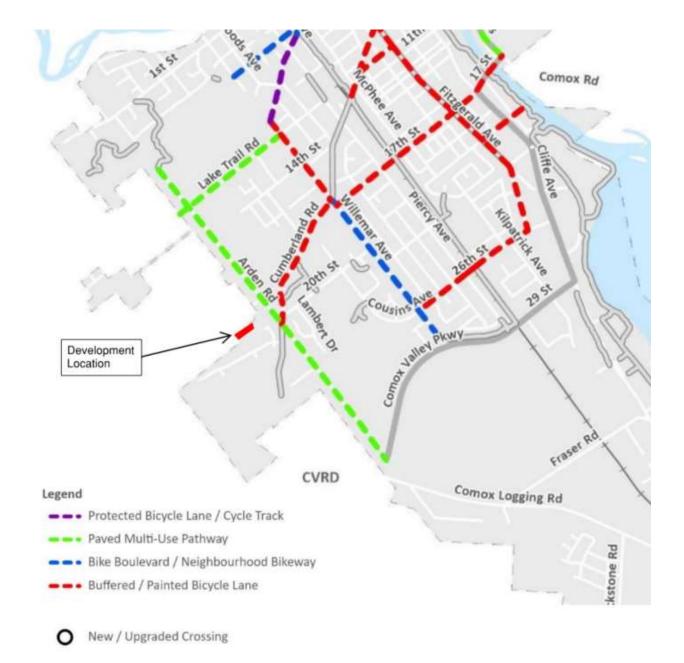


Figure 4: Medium-Term (10 Year) Cycling Improvement Priorities – Transportation Master Plan 2019 (Modified)

### 3.2. Pedestrian Network

There is currently limited sidewalk infrastructure in the vicinity of the development. No sidewalks are provided along Arden Road nor along Lake Trail Road. Sidewalks are available along Cumberland Road along one side of the road. Additionally, there are currently limited protected pedestrian crossings along Cumberland Road and along Lake Trail Road.

As shown in *Figure 5*, based on the City of Courtenay's draft OCP from 2022, paved multi-use pathways adjacent to the street have been recommended along Arden Road and along Lake Trail Road. Additionally, the *Transportation Master Plan* (2019) and the Draft OCP (2022) both identified the Arden Road and Cumberland Road intersection as a desirable location for an improved pedestrian crossing.

Additionally, multiple conceptual multi-purpose trails were proposed in the *Arden Corridor Local Area Plan* (2013) in the vicinity of the development as shown in *Figure 6*.

Recommended Pedestrian Elementary Schools Network Plan Multi-Use Trail (From Secondary Schools Development Parks Plan) Location Hospital Paved Multi-Use Pathway (Adjacent to Street) College Sidewalk **Planned Crossings Existing Network** Improved Crossing Recommended Improved Sidewalk Potential Crossing Crossing Location From Parks Plan

Figure 5: Recommended Pedestrian Network Plan City of Courtenay Draft OCP, 2022 (Modified)



Figure 6: Conceptual Land Use Plan for the Arden Corridor – Arden Corridor Local Area Plan (2013) (Modified)

### 3.3. Transit Network

No BC Transit bus routes are within close walking distance to the development. The nearest bus stop is an approximately 800m walking distance away near 2100 Block of 20<sup>th</sup> Street for BC Transit Route 7 from Arden to Driftwood Mall; however, as noted in the *Arden Corridor Local Area Plan* (2013), although the Arden Corridor is not currently served by transit, this could change in the future when densities are high enough to support service in the area.

# 4. Traffic Volume Development

### 4.1. Existing 2022 Traffic Volumes

Traffic counts for the study intersections were collected by McElhanney between March 29 - 31, 2022, via single day counts from the weekday AM and PM peaks (7 - 9 AM and 3 - 6 PM respectively) for the following intersections:

- · Lake Trail Road and Arden Road,
- Cumberland Road and Arden Road and
- Copperfield Road and Arden Road.

Existing Conditions (2022) Weekday AM and PM peak hour traffic volumes at the study intersections are shown in *Figure 7*. Detailed traffic count sheets are included in *Attachment B*.



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Existing Volumes (2022)

## 4.2. Background Volumes

A 2% annual compound growth rate will be used to project future traffic volumes. This value is consistent with the City of Courtenay's annual population growth projections used in the *Transportation Master Plan* (2019).

A 2025 opening day has been assumed for the purposes of this memo. The opening day (2025) and future opening day + 10 years (2035) Weekday AM and PM peak hour background volumes are presented in *Figure 8* and *Figure 9* respectively.



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Background Volumes (2025)



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Background Volumes (2035)

### 4.3. Trip Generation

Project trip generation refers to the process of estimating the amount of vehicular traffic a development would add to the surrounding roadway system based on land use and development size. For the proposed development, the amount of traffic entering and exiting the road system was calculated for the weekday AM and PM peak hours and the daily totals. For the AM and PM peak hours, the trip rates represent the hourly trips that would occur between the hours of 7:00-9:00 AM and 4:00-6:00 PM, respectively.

Peak hour and daily trip generation estimates for the proposed development were developed using the ITE *Trip Generation*, 10<sup>th</sup> Edition (2017). Peak AM and PM and daily ITE trip generation rates were then applied, as summarized in *Table 1* below.

Table 1: ITE Vehicle Trip Generation Rates

Land Use	Land Use Development		Unit <sup>1</sup>	Veh	icle Trip F	Rate	In / Out Split				
Description	Туре	Code	Onne	AM	PM	Daily	AM (%)	PM (%)	Daily (%)		
Single-Family Detached Housing	Single-Family, Duplex	210	DU	0.74	0.99	9.44	25 / 75	63 / 37	50 / 50		
Multi-Family Housing (Low-Rise)	Townhome	220	DU	0.46	0.56	7.32	23 / 77	63 / 37	50 / 50		

#### Notes:

DU = dwelling units

Note that carriage houses have not been explicitly accounted for in *Table 1*. The ITE *Trip Generation, 10<sup>th</sup> Edition (2017)* does not have a specific trip rate for lots that include both single-family homes and carriage houses. Similarly, specific trip rates are not provided for houses that include additional suites (e.g. basement suites). As a result, it is assumed that these types of land uses have already been incorporated in the trip rates for other land uses, which is in line with the typical methodology used for estimating development related trips.

### 4.3.1. Alternative Transportation Reductions

The City of Courtenay's *Connecting Courtenay – Transportation Master Plan* (Urban Systems, September 2019) states that passenger vehicles account for 85% of all weekday trips made within the City. Walking (8%), cycling (4%) and transit (3%) trips, i.e. sustainable travel modes, account for the remaining 15% of weekday trips. As shown in *Figure 10*, the City has targets to increase the sustainable mode share to 30% in the future. Without transportation investments aimed at improving the transit, walking and cycling infrastructure in the City, the 15% sustainable mode share is expected to continue.

Despite the projected sustainable mode increase, **no reductions for sustainable modes have been assumed** for the traffic analysis conducted in the following sections of the report in order to be conservative.

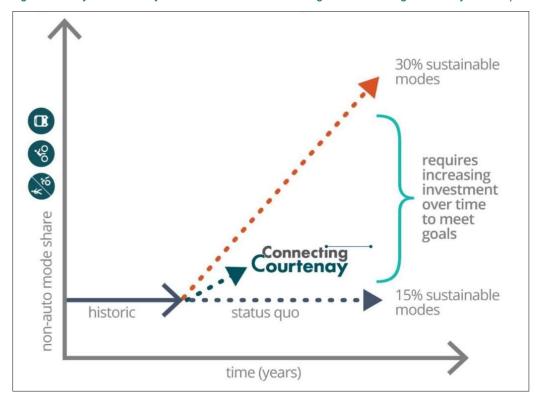


Figure 10: City of Courtenay Sustainable Mode Share Target – Connecting Courtenay – Transportation Master Plan

### 4.3.2. Net Vehicle Trip Generated by Development

*Table 2* presents the net estimated vehicle trips generated from the development using the rates discussed in *Section 4.3*. No reduction has been applied to account for sustainable travel modes.

The proposed development is expected to generate 24 vehicle trips (6 inbound / 18 outbound) and 32 vehicle trips (20 inbound / 12 outbound) during the weekday AM and PM peak hours, respectively. Overall, the development will generate approximately 336 (168 inbound / 168 outbound) total weekday daily trips.

Table 2: Net Site Generated Vehicle Trips

Davidson Toro	Land Use	D	11-24-	# -611-34-	D		Trips	
Development Type	Code	Description	Units	# of Units	Period <sup>1</sup>	ln	Out	Total
		0: 1 5 3			Daily	104	104	208
Single-Family	210	Single-Family Detached Housing	Dwelling Units	22	AM	4	12	16
		Detaction riousing			PM	14	8	22
		0:11 5			Daily	9	9	18
Duplex	210 <sup>2</sup>	Single-Family Detached Housing	Dwelling Units	2	AM	0	1	1
		Detactica floading			PM	1	1	2
					Daily	55	55	110
Townhome	220 <sup>3</sup>	Low-Rise Multifamily Housing	Dwelling Units	15	AM	2	5	7
		Housing			PM	5	3	8
	e (2001)				Daily	168	168	336
	Sub	AM	6	18	24			
					PM	20	12	32

#### Notes:

- 1. AM and PM rates correspond to peak hour of adjacent street traffic
- 2. Trip generation rates for single-family detached housing were used to remain conservative
- 3. Low-rise multifamily housing includes apartments and townhouses with at least three other units

### 4.4. Trip Distribution

The site generated vehicle trips shown in *Table 2* were distributed into the development's nearby road network. The existing 2022 traffic counts were reviewed to help estimate the trip distribution and assignment for the site generated trips for the weekday AM and PM peak hours. The majority of trips are expected to head to / from Downtown Courtenay to the northeast while a smaller portion of trips are expected to head south towards the highway. Additionally, a portion of trips are expected to head west along Lake Trail Road to go to Arden Elementary.

The trip distribution assumptions are listed below and are shown in Figure 11.

- o 10% of traffic to / from west along Lake Trail Road,
- o 20% of traffic to / from east along Lake Trail Road,
- o 20% of traffic to / from west along Cumberland Road and
- o 50% of traffic to / from east along Cumberland Road.



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FIGURE 11

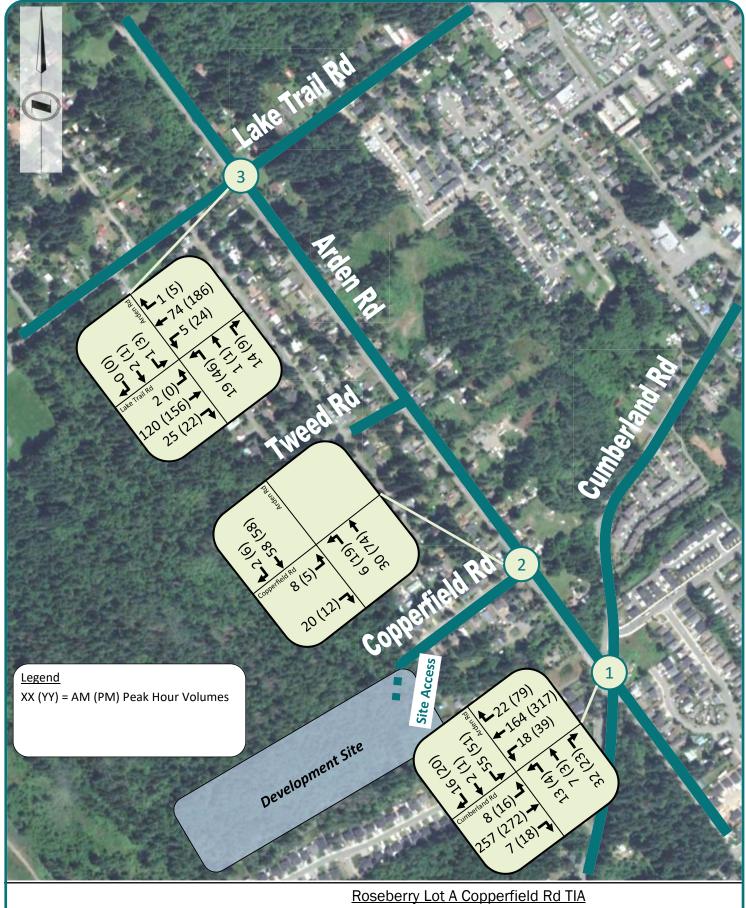
Not to Scale

### 4.5. With Project Volumes

To determine traffic volumes for Opening Day (2025) and Future Year (2035) with the development (i.e. combined), the development trips estimated for the project development (*Table 2*) were added to the background traffic for each of the study scenarios. The development generated traffic volumes are shown in *Figure 12* while the combined Weekday AM and PM peak hour traffic volumes are presented in *Figure 13* and *Figure 14* for the 2025 and 2035 post-development conditions, respectively.



Site Generated Trips



Combined Volumes (2025)



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Combined Volumes (2035)

## **5. Traffic Operational Analysis**

Traffic operations analysis was conducted for two scenarios:

- Existing (2022)
- Opening Day (2025)
- Future Year (2035)

All scenarios were analyzed for two time periods:

- Weekday AM peak hour
- Weekday PM peak hour

All future scenarios (2025 and 2035) were analyzed with and without the additional trips generated by the proposed development.

### 5.1. Synchro Analysis Software

Synchro software, version 10.0, was used to report the Level of Service (LOS) and average delay at each of the study intersections. Synchro is a traffic software package used to determine traffic conditions based on volumes, laning, and type of traffic control. Synchro calculates average delays and queue lengths for each movement at an intersection. Average delays are then translated into LOS. Detailed Synchro analysis reports can be found in *Attachment C*.

#### 5.2. Intersection Level of Service Criteria

Operations of roadway facilities are described in terms of Level of Service (LOS). LOS is a qualitative description of traffic flow based on factors such as speed, travel time, delay, and freedom to manoeuvre. For intersections Level of Service is based on delay. Six service levels are defined, ranging from LOS A, the best operating conditions, to LOS F, the worst operating conditions. LOS E corresponds to "at or near capacity" operations. When volumes exceed capacity, it results in stop-and-go conditions, which is designated as LOS F. The delay thresholds and corresponding LOS are presented in *Table 3*. The typical criterion for acceptable operation is LOS D. Therefore, any movement or intersection operating at LOS E or worse may require improvement.

Table 3: Intersection Level of Service Definitions

Level of	Delay Criter	ia (sec / veh)	
Service	Signalized Intersections <sup>1</sup>	Unsignalized Intersections <sup>2</sup>	Description
А	<u>&lt;</u> 10	<u>&lt;</u> 10	Represents free flow. Individual users are virtually unaffected by others in the traffic stream. Usually no conflicting traffic.
В	> 10 to 20	> 10 to 15	Stable flow, but the presence of other users in the traffic stream begins to be noticeable. Occasionally some delay due to conflicting traffic.
С	> 20 to 35	> 15 to 25	Stable flow, but the operation of individual users becomes significantly affected by interactions with others in the traffic stream. Delay is noticeable, but not inconveniencing.
D	> 35 to 55	> 25 to 35	Represents high-density, but stable flow. Delay is noticeable and irritating; increased likelihood of risk taking.
E	> 55 to 80	> 35 to 50	Represents operating conditions at or near the capacity level. Delay approaching tolerance levels; risk taking behaviour is likely.
F	> 80	> 50	Represents forced or breakdown flow. Delay exceeds tolerance level; high likelihood of risk taking.

#### Notes:

Values shown are n seconds / vehicle. **BOLD** indicates unacceptable LOS.

- 1. HCM 6<sup>th</sup> Edition
- 2. HCM 6<sup>th</sup> Edition

For unsignalized (side-street stop-controlled and 4-way stop controlled) intersections, the LOS calculations were conducted based on Synchro's default capacity analysis methodology, which correspond with the methodology from HCM 2000. The LOS rating is based on the average delay expressed in seconds per vehicle. For controlled approaches composed of a single lane, the control delay is computed as the average of all movements in that lane.

It should be noted that although Synchro reports overall intersection LOS at side-street stop-controlled unsignalized intersections, the overall LOS is not a good indicator of the side street performance, as it is calculated from the average delay for all vehicles. As a result, the overall LOS is typically heavily skewed toward the LOS for the free flow major movement, particularly where the proportion of free flow volume on the major street is very high.

### 5.3. Level of Service Results

### Existing Conditions (2022)

Analysis was conducted for the Existing (2022) conditions. A summary of the Existing (2022) LOS results can be found in *Table 4*. The detailed results can be found in *Attachment C*. For the purposes of the analysis for the Existing (2022) as well as all future scenarios, Arden Road is assumed to be a north-south road. All other roads have been considered east-west.

For existing conditions, all intersection movements are expected to operate at LOS C or better.

Table 4: Existing (2022) – Intersection Level of Service Results

					20	22 (Exis	ting Co	nditions	;)						
Int.	Time	Attribute	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
		Volume	7	242	7	17	155	18	12	7	30	43	2	11	
		v/c Ratio		0.01			0.02			0.12		0.19			
	AM	Delay (s)		0			1			13			17		
		LOS		Α			Α			В			С		Α
Cumberland Rd and		95% Q (m)		0			1			3			6		
Arden Rd		Volume	11	256	17	37	299	65	4	3	22	42	1	17	
		v/c Ratio		Α			A			В			С		
	PM	Delay (s)		0		1				13			21		3
		LOS		Α		Α				В			С		Α
	95% C (m)			0			1			3			9		
		Volume	3		7				2	28			55	1	
	v/c Ratio		0.01						0			0.04			
	AM	Delay (s)		9					1			0			1
		LOS		Α						Α			Α		Α
Copperfield Rd and		95% Q (m)	0							0			0		
Arden Rd		Volume	1		4				5	70			55	1	
		v/c Ratio		0.01					0			0.05			
	PM	Delay (s)		9					1			0			1 A
		LOS		Α						Α			A		
		95% Q (m)		0						0			0		
		Volume	2	113	23	4	70	1	16	1	9	1	2	1	
		v/c Ratio		0			0			0.05			0.01		
	AM	Delay (s)		0			0			10			10		2
		LOS		Α			Α			Α			В		Α
Lake Trail		95% Q (m)		0			0			1			0		
Arden Rd	Rd and	Volume	1	147	19	19	175	5	42	1	7	3	1	1	
		v/c Ratio		0			0.02			0.13			0.03		
	PM	Delay (s)		0			1		13			12			2
		LOS		Α			Α		В			В			Α
		95% Q (m)		0			0			4			1		

### Opening Day (2025)

For Opening Day (2025) conditions, the analysis was conducted with the background traffic only, and again with the inclusion of the project development traffic. A summary of the Opening Day (2025) LOS results for the background scenario and with development scenario can be found in *Table 5* and *Table 6*. The detailed results can be found in *Attachment C*.

For the opening day **background scenario**, all intersection movements are expected to operate at LOS C or better.

For the opening day **post development scenario**, the southbound Arden Road approach at Cumberland Road is expected to deteriorate from LOS C to LOS D in the PM peak period. However, it is noted that the development traffic (an additional 8 vehicles per hour) is only expected to add 2 seconds of additional delay. All other intersection movements are expected to operate at LOS C or better.

Table 5: Opening Day (2025) – **Background** - Intersection Level of Service Results

					2025	(Backg	round C	onditio	ns)						
Int.	Time	Attribute	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
		Volume	7	257	7	18	164	19	13	7	32	46	2	12	
		v/c Ratio		0.01			0.02		0.13				0.22		
	AM	Delay (s)		0		1				13			18		
		LOS		Α			Α			В			С		Α
Cumberland Rd and		95% Q (m)		0			1			4			7		
Arden Rd		Volume	12	272	18	39	317	69	4	3	23	45	1	18	
		v/c Ratio		0.01			0.03			0.10			0.32		
	PM	Delay (s)		0			1			13			24		4
		LOS		Α			Α			В			С		Α
		95% Q (m)		0			1			3			11		
		Volume	3		7				2	30			58	1	
		v/c Ratio		0.01						0			0.05		
	AM	Delay (s)		9					1			0			1
		LOS		Α						A			A		
Copperfield Rd and		95% Q (m)	0							0			0		
Arden Rd		Volume	1		4				5	74			58	1	
		v/c Ratio		0.01					0				0.05		
	PM	Delay (s)		9					1			0			1
		LOS		Α						A			Α		Α
		95% Q (m)		0						0			0		
		Volume	2	120	24	4	74	1	17	1	10	1	2	0	
		v/c Ratio		0			0			0.05			0.01		
	AM	Delay (s)		0			0			10			11		2
		LOS		Α			Α			Α			В		Α
Lake Trail Rd and		95% Q (m)		0			0			1			0		
Arden Rd		Volume	1	156	20	20	186	5	45	1	7	3	1	1	
	Aldell Ita	v/c Ratio		0			0.02			0.14		0.03			
	PM	Delay (s)		0			1		14			12			2
		LOS		Α			Α			В			В		Α
		95% Q (m)		0			0			4			1		

Table 6: Opening Day (2025) - With Development - Intersection Level of Service Results

					202	5 (Coml	oined Co	ondition	s)						
Int.	Time	Attribute	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
		Volume	8	257	7	18	164	22	13	7	32	55	2	16	
		v/c Ratio		0.01			0.02			0.13		0.27			
	AM	Delay (s)		0			1		13			18			4
		LOS		Α			Α			В			С		Α
Cumberland Rd and		95% Q (m)		0			1 4				9				
Arden Rd		Volume	16	272	18	39	317	79	4	3	23	51	1	20	
		v/c Ratio		0.02		0.03				0.10			0.37		
	PM	Delay (s)		0		1				13			26		4
	LOS		A				Α			В			D		Α
		95% Q (m)		0			1			3			13		
		Volume	8		20				6	30			58	2	
		v/c Ratio		0.04						0.01			0.05		
	AM	Delay (s)		9					1			0			2
		LOS		Α						Α		A			Α
Copperfield Rd and		95% Q (m)	1							0			0		
Arden Rd		Volume	5		12				19	74			58	6	
		v/c Ratio		0.03					0.02			0.06			
	PM	Delay (s)		9					2			0			2
		LOS		Α					Α				Α		Α
		95% Q (m)		1						0			0		
		Volume	2	120	25	5	74	1	19	1	14	1	2	1	
		v/c Ratio		0			0			0.06			0.01		
	AM	Delay (s)		0			1			10			10		2
		LOS		Α			Α			Α			В		Α
Lake Trail Rd and		95% Q (m)		0			0			2			0		
Arden Rd		Volume	1	156	22	24	186	5	45	1	8	3	1	1	
	Aldell Nu	v/c Ratio		0			0.02		0.15			0.03			
	PM	Delay (s)		0			1			14			12		3
		LOS		Α			Α			В			В		Α
		95% Q (m)		0			1			4			1		

#### Future Conditions (2035)

For the Future Year (2035) conditions, the analysis was conducted with the background traffic only, and again with the inclusion of the project development traffic. A summary of the Future Year (2035) LOS results is attached in *Table 7* and *Table 8*. The detailed results can be found in *Attachment C*.

For the Future Year (2035) background scenario:

- At Cumberland Road and Arden Road, the southbound Arden Road approach is expected to deteriorate to LOS C in the AM peak and LOS E in the PM peak. As noted previously, the Transportation Master Plan contemplates a signal or roundabout at this location in the future which should address this LOS E.
- All other movements are expected to operate at LOS C or better.

With the addition of development traffic for the 2035 scenario:

- At Cumberland Road and Arden Road, the southbound approach is expected to deteriorate from LOS C to LOS D in the AM peak and remain at LOS E in the PM peak. Queues are expected to remain manageable, with 95<sup>th</sup> percentile queues of 28m, or approximately 4 passenger vehicles, in the worst-case PM peak.
- All other movements are expected to operate at LOS C or better.

Table 7: Future Year (2035) – Background - Intersection Level of Service Results

					2035	(Backg	round C	Conditio	ns)						
Int.	Time	Attribute	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
		Volume	9	313	9	22	200	23	16	9	39	56	2	15	
		v/c Ratio		0.01			0.03			0.19			0.35		
	AM	Delay (s)		0		1				15			24		
		LOS		Α			Α			С			С		Α
Cumberland Rd and		95% Q (m)		0			1			6			12		
Arden Rd		Volume	15	332	22	48	386	84	5	4	28	55	1	22	
		v/c Ratio		0.02			0.04			0.15			0.53		
	PM	Delay (s)		1			1			15			41		5
	LO		А				Α			С			Е		Α
		95% Q (m)	0				1			4			22		
		Volume	4		9				2	37			71	1	
		v/c Ratio		0.02						0			0.06		
	AM	Delay (s)		9					0			0			1
		LOS		Α						A			A		
Copperfield Rd and		95% Q (m)		0						0			0		
Arden Rd		Volume	1		5				6	90			71	1	
		v/c Ratio		0.01					0				0.07		
	PM	Delay (s)		9					1			0			1
		LOS		Α					Α			A			Α
		95% Q (m)		0						0			0		
		Volume	2	146	29	5	90	1	21	1	12	1	2	1	
		v/c Ratio		0			0			0.07			0.01		
	AM	Delay (s)		0			0			10			11		2
		LOS		Α			Α			В			В		Α
Lake Trail		95% Q (m)		0			0			2			0		
Arden Rd	Rd and Arden Rd	Volume	1	190	24	24	227	6	55	1	9	4	1	1	
		v/c Ratio		0			0.02			0.2		0.04			
	PM	Delay (s)		0			1		16			14			3
		LOS		Α			Α			С			В		Α
		95% Q (m)		0			1			6			1		

Table 8: Future Year (2035) - With Development - Intersection Level of Service Results

					203	5 (Coml	oined Co	ondition	s)						
Int.	Time	Attribute	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
		Volume	10	313	9	22	200	26	16	9	39	65	2	19	
		v/c Ratio		0.01			0.03			0.2			0.41		
	AM	Delay (s)		0			1		15				26		
		LOS		Α		Α				С			D		Α
Cumberland Rd and		95% Q (m)		0			1			6			15		
Arden Rd		Volume	19	332	22	48	386	94	5	4	28	61	1	24	
		v/c Ratio		0.02			0.04			0.15			0.61		
	PM	Delay (s)		1		1				16			48		6
		LOS		Α			Α			С			E		Α
		95% Q (m)	0				1			4			28		
		Volume	9		22				6	37			71	2	
		v/c Ratio		0.04					0.01			0.06			
	AM	Delay (s)		9					1			0			2
		LOS		Α						Α			Α		
Copperfield Rd and		95% Q (m)	1							0			0		
Arden Rd		Volume	5		13				20	90			71	6	
		v/c Ratio		0.03					0.02			0.07			
	PM	Delay (s)		9					2			0			2
		LOS		Α						Α			Α		
		95% Q (m)		1						0			0		
		Volume	2	146	30	6	90	1	23	1	16	1	2	1	
		v/c Ratio		0			0.01			0.08			0.01		
	AM	Delay (s)		0			1			10			11		2
		LOS		Α			Α			В			В		Α
Lake Trail		95% Q (m)		0			0			2			0		
Arden Rd	Rd and	Volume	1	190	26	28	227	6	56	1	11	4	1	1	
		v/c Ratio		0			0.03			0.22			0.04		
	PM	Delay (s)		0			1		16			14			3
		LOS		Α			Α		С			В			Α
		95% Q (m)		0			1			7			1		

# 6. Site Access

### 6.1. Description

The development site has proposed main entrance at the west end of Copperfield Road. Copperfield Road currently at the site. It has been proposed that Copperfield Road will be terminated at a "hammerhead" turnaround near the site access as shown in *Figure 15*.

Figure 15: Proposed Development Site Access



# 7. Safety Analysis

ICBC collision data from ICBC's Tableau Public crash map from 2016 – 2020 was reviewed at the study intersections as shown in *Table 9*. The ICBC map classifies collisions into two categories:

- Casualty crashes are crashes resulting in injury or fatality.
- Property damage only (PDO) crashes are crashes resulting in material damage and no injury or fatality.

The publicly accessible ICBC data does not provide any additional information on the crashes beyond the number and classification.

Based on the ICBC collision data, Cumberland Road and Arden Road was found to have the highest number of collisions (21) of the three study intersections. It is noted that:

- For the Cumberland Road / Arden Road intersection, bike lane and pavement marking upgrades were installed some time after August 2019 based on Google Earth historical imagery. Looking only at the collisions from 2019 – 2020, a total of 6 collisions occurred, with 4 PDO collisions and 2 casualties.
- As indicated in the City of Courtenay's Draft OCP (January 2022) and the Arden Corridor Local
  Area Plan (2013), further intersection improvements may be installed at Arden Road and
  Cumberland Road. Options considered in the plans include the potential for a roundabout at the
  intersection, which may mitigate any existing safety issues.

Table 9: ICBC Collision Data - 2016 - 2020

Intersection	Casualty	Property Damage Only	Total
Cumberland Rd and Arden Rd	6	15	21
Copperfield Rd and Arden Rd	0	1	1
Lake Trail Rd and Arden Rd	1	2	3

## 8. Conclusion and Recommendation

### 8.1. Conclusion

The purpose of this letter report was to review the traffic operations for the proposed residential developments at 2650 Copperfield Road in Courtenay, BC.

### **Trip Generation**

The proposed developments are expected to generate 24 and 32 new vehicle trips per hour in the weekday AM and PM peak periods respectively. A total of 336 trips are expected to be generated per weekday.

#### Traffic Analysis

Traffic analysis was conducted for the study intersections during the weekday AM and PM peak hour periods. Analysis was also conducted for three horizon years, the Existing (2022), Opening Day (2025) and Future Year (2035).

### Existing (2022)

For Existing (2022) conditions, all intersection movements operate at LOS C or better.

### Opening Day (2025)

During the Opening Day (2025) scenario all intersection movements are expected to operate at LOS D or better with and without development traffic.

### Future Year (2035)

During the Future Year (2035) scenario, the southbound Arden Road approach of Cumberland Road / Arden Road is expected to deteriorate to LOS E in the background case PM peak. With the addition of development traffic, the southbound Arden Road approach is expected to remain within LOS E. Despite the LOS E performance, queues are expected to be manageable, with 95th percentile queues of 28m (~4 passenger cars) for the "with development" case. All other intersection movements are expected to perform at LOS D or better with and without development traffic in 2035. The Transportation Master Plan contemplates a signal our roundabout at the Cumberland Road/Arden Road intersection, which would improve the Level of Service.

#### Site Access

No operational or safety concerns were identified at the site access along Copperfield Road.

### Safety Analysis

Collision data collected over a 5-year period, from 2016 to 2020, was reviewed at the study intersections. A total of 21 collisions were recorded at Cumberland Road / Arden Road, 1 collision at Copperfield Road / Arden Road, and 3 collisions at Lake Trail Road / Arden Road.

### 8.2. Recommendation

Given the acceptable performance of the study intersections throughout each time horizon with the addition of development traffic, improvements to the intersections are not expected to be required.

# 9. Closing

If you have any questions or concerns regarding this analysis, please contact the undersigned.

Yours truly,

McElhanney Ltd.

Prepared by:

604-424-4803

Reviewed by:

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Traffic Engineer
Traffic & Road Safety Division
dsu@mcelhanney.com

Tavisla

Mark Merlo, P.Eng., PTOE Senior Transportation Engineer Traffic & Road Safety Division mmerlo@mcelhanney.com 236-317-5830

Attachments: A – Development Site Plan

B – Traffic Count Data C – Synchro Reports

D - Statement of Limitations

CC: Neil Penner, McElhanney Ltd.

Andy Gaylor, McElhanney Ltd. Bob Bigelow, McElhanney Ltd.

## **ATTACHMENT A**

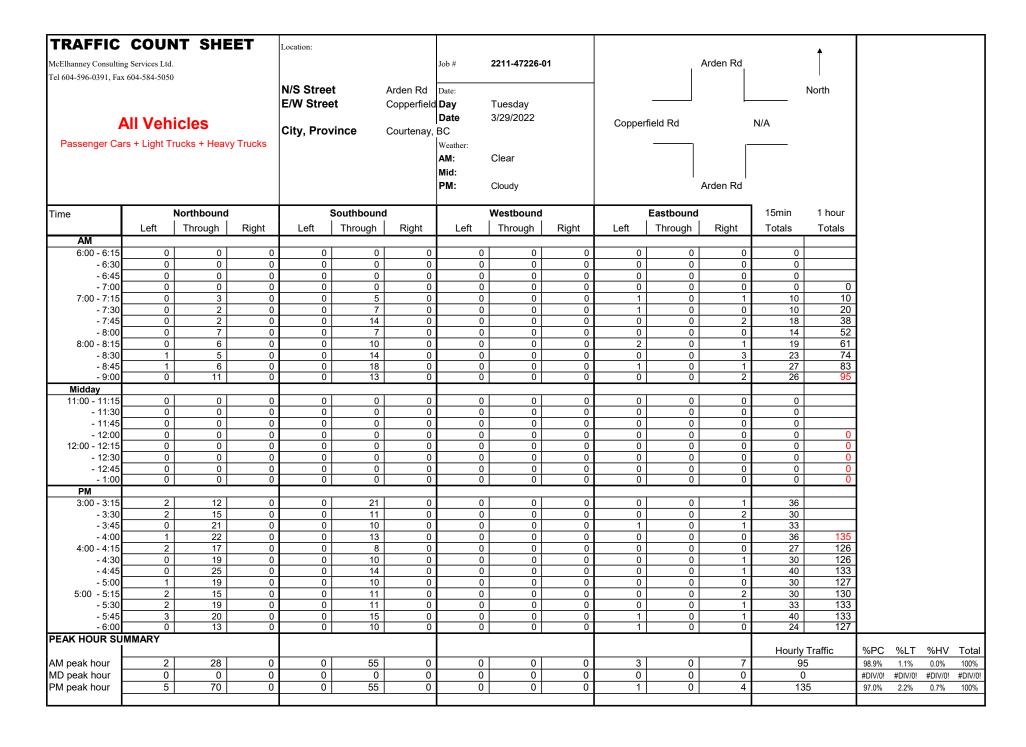
Site Plan



## Vehicular and Pedestrian Movement Plan

## ATTACHMENT B

**Traffic Counts** 



TRAFFIC	COU	NT SHE	ET	Location:										
McElhanney Consultin				Location			Job #	2211-47226-	01			Arden Rd		1
Tel 604-596-0391, Fax							,							
				N/S Stree		Arden Rd	Date:						I	North
				E/W Stree	et	Copperfield	-	Tuesday				_		
Pas	ssenge	er Cars		City, Prov	vinco	Courtonav	Date	3/29/2022		Copper	field Rd		N/A	
				City, Fio	VIIICE	Courtenay,	Weather:					-		
							AM:	Clear						
							Mid:							
							PM:	Cloudy				Arden Rd		
Time		Northbound			Southboun	d		Westbound			Eastbound		15min	1 hour
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
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- 6:45													0	
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- 7:45		2			14					<u> </u>		2	18	38
- 8:00		7			6								13	51
8:00 - 8:15		6			10					2		1	19	60
- 8:30 - 8:45	<u>1</u>				14 17		<b>-</b>			1		3	23 26	73 81
- 9:00		11			13							2	26	94
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<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

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McElhanney Consultin	ng Services Lto	i.					Job #	2211-47226-	01			Arden Rd		Ī
Tel 604-596-0391, Fa	x 604-584-505	0												
				N/S Stree		Arden Rd Copperfield	Date:	Tuesday				L		North
	ight T	rucke				Ооррегиен	Date	3/29/2022		Connei	rfield Rd		N/A	
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							PM:	Cloudy				Arden Rd		
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<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHE	EET	Location:										<u> </u>
McElhanney Consultin							Job #	2211-47226-	01		1	Arden Rd		Ī
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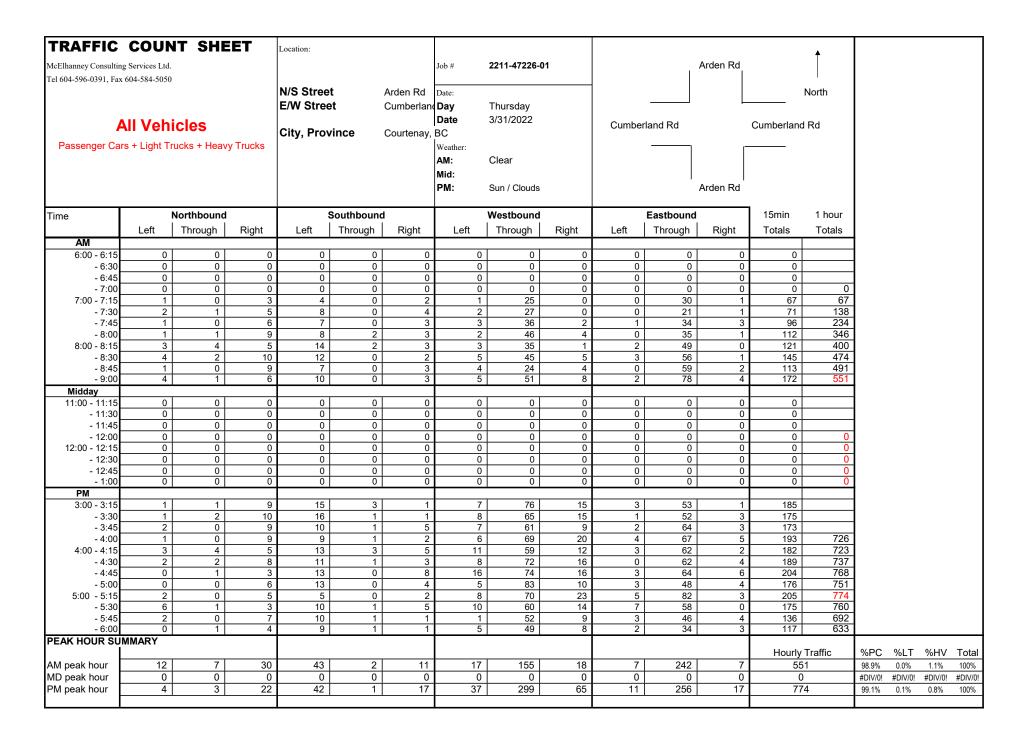
<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

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McElhanney Consulting					Location:				Job #	2211-47226	-01					Arden Rd		<b>↑</b>
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<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

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McElhanney Consultin	g Services Ltd.						Job #	2211-47226-	01			Arden Rd		Ī
Tel 604-596-0391, Fax														
				N/S Stree		Arden Rd	Date:	_						North
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- 6:00 PEAK HOUR SU	MMARY 1.	1			1						<u> </u>			0
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<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles



TRAFFIC	COUN	IT SHE	ET	Location:										
McElhanney Consultin	g Services Ltd.						Job #	2211-47226-	01			Arden Rd		1
Tel 604-596-0391, Fax	k 604-584-5050													ı
				N/S Stree		Arden Rd	Date:	Th						North
_				E/W Stree	et.	Cumberlan	Date	Thursday 3/31/2022						
Pas	ssenge	er Cars		City, Prov	/ince	Courtenay,	•	0/01/2022		Cumber	rland Rd		Cumberland	Rd
				•		•	Weather:					Г		
							AM:	Clear						
							Mid: PM:	Sun / Cloud			I	 Arden Rd		
							PIVI:	Sun / Cloud	S			Arden Ka		
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	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
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- 7:45	1		6	7		3	3	34	2	1	33	2	92	219
- 8:00	1	1	9	7	2	3	2	45	4		33	1	108	327
8:00 - 8:15 - 8:30	2	4 2	5 10	14 11	2	3 2	3 5	32 45	1 5	3	49 56	1	117 144	384 461
- 8:45	1		9	7		3	4	24	4		59	2	113	482
- 9:00	4	1	6	10		3	5	51	8	2	77	4	171	545
Midday 11:00 - 11:15													0	
- 11:30													0	
- 11:45						I							0	
- 12:00 12:00 - 12:15													0	0
- 12:30													0	0
- 12:45 - 1:00													0	0
PM											1		0	- 0
3:00 - 3:15	1	1	9	15	3	1	7	75	15	3	52	1	183	
- 3:30 - 3:45	1 2	2	10 9	16 10	1	<u>1</u> 5	8 7	64 60	15 9	1 2	50 59	3	172 167	
- 4:00	1		9	8	1	2	6	67	19	4	67	5	189	711
4:00 - 4:15	2	4	5	13	3	5	11	58	12	3	61	2	179	707
- 4:30 - 4:45	2	2	8	9	1	3 8	8 16	72 73	15 16	3	62 64	6	186 203	721 757
- 4:45 - 5:00		1	6	13		4	5	82	10	3	48	4	175	743
5:00 - 5:15	2		5	5		2	8	70	23	5	81	2	203	767
- 5:30 - 5:45	6	1	7	10 10	1	5	10	59 52	14 9	7	58 45	4	174 135	755 687
- 6:00		1	4	9	1	1	5		8	2	34	3	117	629
PEAK HOUR SU	MMARY 1.	'									'			
AM pook bour	11	7	20	40		44	47	150	40		244	7	Hourly	
AM peak hour MD peak hour	<u>11</u>	7 0	30	42 0	0	11	17	152 0	18 0	7	241	7	545 0	)
PM peak hour	4	3	22	40	1	17	37	297	64	11	255	16	767	7
1. Pook bour volu											· '			

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

Left   Through   Right   Left   Through   Right   Left   Through   Right   Left   Through   Right   Totals   Totals	TRAFFIC	COU	NT SHI	EET	Location:										
North   Nort								Job #	2211-47226-	01		1	Arden Rd		Ī
PM: Sun / Clouds					E/W Stree	et	Cumberlan	Day Date BC Weather: AM:	3/31/2022		Cumbe	rland Rd	[		
Left									Sun / Cloud	S		'	Arden Rd		
Left	Time		Northbound	t		Southbound	d		Westbound			Eastbound		15min	1 hour
## AM		Left			•		1	Left			Left		1	Totals	Totals
-6:30										-					
- 6-45															
-7-00 7-00-7-15 -7-30 -7-45 -7-45 -8-00 -8-00 -8-00 -8-00 -8-45 -8-30 -8-45 -1-100 -11-15 -11-100 -11-15 -11-100 -11-15 -11-100 -11-10								<u> </u>							
-7:30															0
- 7.45															0
B.00															0
8:00 - 8:15					1										<u>0</u>
- 8:30					<u> </u>										1
-9:00															1
Midday															1
11:00 - 11:15														0	0
- 11:30			1	<u> </u>				<b></b>				Ī			
- 11:45															
12:00 - 12:15															
- 12:30															0
- 12:45															0
-1:00   PM															0
PM															0
- 3:30															
- 3:45															
- 4:00															
4:00 - 4:15       0         - 4:30       1         - 4:45       0         - 5:00       0         5:00 - 5:15       0         - 5:30       0         - 5:45       0         - 6:00       0         PEAK HOUR SUMMARY 1.         AM peak hour MD pe			+		1			-					-		1
- 4:30					<del>                                     </del>			<b> </b>							1
- 4:45					1										2
5:00 - 5:15 - 5:30 - 5:45 - 6:00  PEAK HOUR SUMMARY 1.  AM peak hour MD peak hour 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	- 4:45													0	2
- 5:30															1
- 5:45															1 0
-6:00															0
PEAK HOUR SUMMARY 1.           AM peak hour MD peak hour 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					<u> </u>										0
AM peak hour 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	PEAK HOUR SU	MMARY 1.	•	•		•	•					•		Hourly	Traffic
MD peak hour 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	AM peak hour	n	n	n	n	0	n	n	n	n	0	0	l n		
·															

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHE	EET	Location:										
McElhanney Consultir	ng Services Ltd	1.					Job #	2211-47226-	01		ı	Arden Rd		Ť
Tel 604-596-0391, Fa:		rucks		N/S Stree E/W Stree City, Prov	et	Arden Rd Cumberlan Courtenay,	Date	Thursday 3/31/2022 Clear Sun / Cloud	İs	Cumbei	rland Rd	Arden Rd	Cumberland	North I Rd
Time		Northbound	l		Southbound			Westbound	1		Eastbound		15min	1 hour
Time	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
AM	2011				111104911	1							· otalio	
6:00 - 6:15													0	
- 6:30													0	
- 6:45 - 7:00													0	0
7:00 - 7:15											7		7	7
- 7:30								2			2		4	11
- 7:45								2			1	1		15
- 8:00 8:00 - 8:15	1						-	3			2		3 4	18 15
- 8:30				1				3					1	12
- 8:45				<u>'</u>									0	8
- 9:00											1		1	6
Midday					1	1								
11:00 - 11:15 - 11:30													0	
- 11:30 - 11:45		+											0	
- 12:00													0	0
12:00 - 12:15													0	0
- 12:30													0	0
- 12:45 - 1:00													0	0
PM													0	
3:00 - 3:15								1			1		2	
- 3:30								1			2		3	
- 3:45								1			5		6	
- 4:00 4:00 - 4:15	1							2	1		1		3	14 15
4:00 - 4:15 - 4:30	1			1				1	1		1		2	14
- 4:45				<u> </u>				1	'				1	9
- 5:00								1					1	7
5:00 - 5:15											1	1		6
- 5:30		-					ļ	1			1		1	5
- 5:45 - 6:00				<del>                                     </del>			<del>                                     </del>			<b>-</b>	1		1 0	<u> </u>
PEAK HOUR SU	MMARY 1.	1			1			1			ı		<u>'</u>	
AAA				ļ .							1 -		Hourly	
AM peak hour MD peak hour	1 0		0	1 0	0	0	0	3	0	0	1 0	0		
PM peak hour	0		0	1	0	0	0		0	0	1	1		
i wi peak noul		1 0	0	<u> </u>	1 0	1 0	- ·		<u> </u>	- ·	1		1	
<u> </u>														

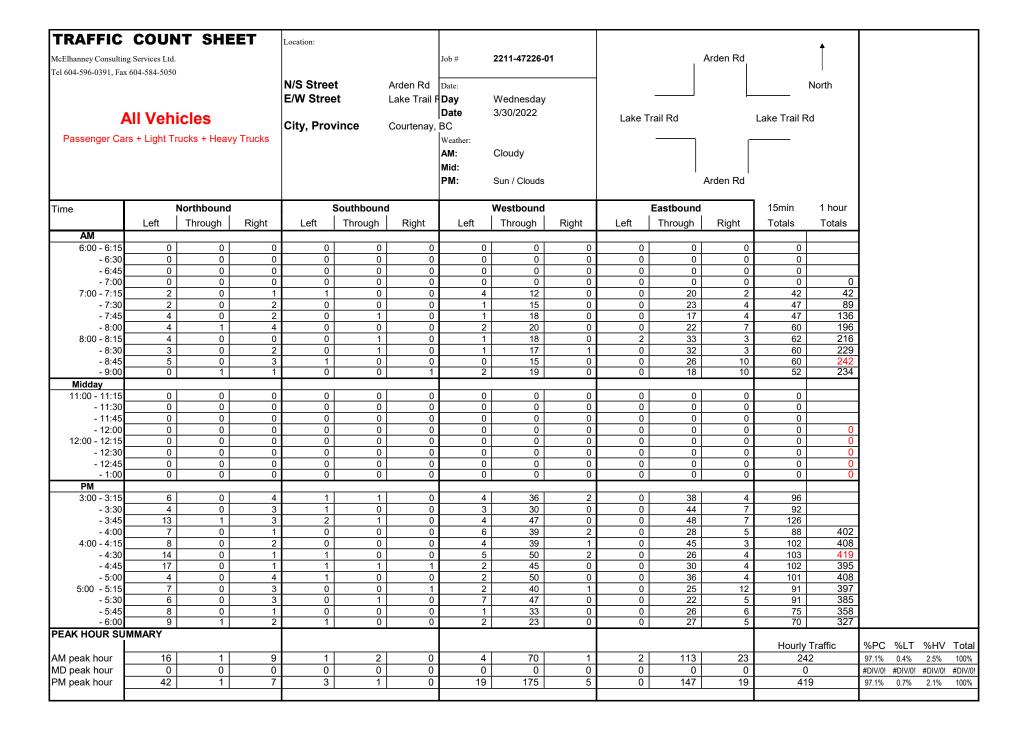
<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TD A EE IO	001111	<b>T</b> 0111																
TRAFFIC	COUN	1 2HI	EEI		Location:													<b>+</b>
McElhanney Consulting	g Services Ltd.								Job #	2211-47226	-01					Arden Rd		
Tel 604-596-0391, Fax	604-584-5050																	ı
					N/S Stree			Arden Rd	Date:									North
					E/W Stre	et		Cumberlan	_	Thursday						-		
	Ped	lestriai	n						Date	3/31/2022			Cumber	land Rd			Cumberland	l Rd
		Cotilai	•		City, Pro	vince		Courtenay,	1									
									Weather:									
									AM:	Clear								
									Mid:									
									PM:	Sun / Cloud	ls					Arden Rd		
					-	N								100				
Time	I		h Leg	1			h Leg	1		1	t Leg	1	<b>.</b>		st Leg		15min	1 hour
	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Totals	Totals
<b>AM</b> 6:00 - 6:15				1						T	1						0	
- 6:30										+							0	
- 6:45																	0	
- 7:00																	0	0
7:00 - 7:15		1				2											3	3
- 7:30 - 7:45					-					+							0	3
- 7:45 - 8:00										1				1			2	5
8:00 - 8:15										·				·			0	2
- 8:30		1															1	3
- 8:45							1										1	4
- 9:00 <b>Midday</b>							1										1	3
11:00 - 11:15																	0	
- 11:30																	0	
- 11:45																	0	
- 12:00																	0	0
12:00 - 12:15 - 12:30					1					+							0	0
- 12:45																	0	0
- 1:00																	0	0
PM																		
3:00 - 3:15				1						-							0	
- 3:30 - 3:45			4			3				-							7	
- 4:00			4			- 3				+				1			1	8
4:00 - 4:15																	0	8
- 4:30						1				1							2	10
- 4:45		1				1				<del> </del>	_						2	5
- 5:00					-		1			2	2						2	8 10
5:00 - 5:15 - 5:30		3							1	+							3	11
- 5:45		1								2				1			4	13
- 6:00		2								1							3	12
PEAK HOUR SU	EAK HOUR SUMMARY <sup>1.</sup>																	
l					<del> </del>	-	-						_	-		_		Traffic
AM peak hour MD peak hour	0	1					2		0				0	0		0	3	
PM peak hour	0	0					0		0		3		0	0		0	11	
I W poak flour	M peak hour 0 1 0				†				т				0		1 0	<u>_</u>		_
<u> </u>																		

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHE	ET	Location:										_
McElhanney Consultin	g Services Ltd	l.					Job #	2211-47226-	01			Arden Rd		1
Tel 604-596-0391, Fax	k 604-584-505	0												ı
				N/S Stree		Arden Rd	Date:	<b>-</b>						North
				E/W Stree	et	Cumberlan	Day Date	Thursday 3/31/2022						
	Bicyc	clist		City, Prov	vince	Courtenay,		3/3 1/2022		Cumber	rland Rd		Cumberland	Rd
				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		, , , , , , , , , , , , , , , , , , ,	Weather:					ľ		
							AM:	Clear						
							Mid:	0 (0)						
							PM:	Sun / Cloud	S			Arden Rd		
Time		Northbound			Southbound	d		Westbound			Eastbound		15min	1 hour
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
AM					1									
6:00 - 6:15 - 6:30													0	
- 6:45													0	
- 7:00 7:00 - 7:15											2		0 2	0
- 7:30											5		5	7
- 7:45 - 8:00													0	7
8:00 - 8:15													0	5
- 8:30									3		1		4	4
- 8:45 - 9:00				1		1		2			2		2 4	6 10
Midday														-
11:00 - 11:15 - 11:30													0	
- 11:45													0	
- 12:00													0	0
12:00 - 12:15 - 12:30													0	0
- 12:45													0	0
- 1:00 <b>PM</b>				-									0	0
3:00 - 3:15								1					1	
- 3:30 - 3:45		1	1	3				1					6	
- 4:00		1			1			2			1		5	12
4:00 - 4:15	1			1				1					3	14
- 4:30 - 4:45				-	1			2	1		1 2		5	10 15
- 5:00													0	10
5:00 - 5:15 - 5:30				-				1 1		1			2	9
- 5:45								2					2	5
- 6:00	BARA A DV 1							3					3	8
PEAK HOUR SU	MMARY "												Hourly	Traffic
AM peak hour	0		0	1	0	1	0	2	3	0	3	0	10	
MD peak hour	0		0	0	0	0	0	0	0	0	0	0		
PM peak hour	0	0	0	0	1	0	0	3	1	1	3	0	9	
<u></u>														

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles



TRAFFIC	COUN	IT SHE	ET	Location:										
McElhanney Consultin							Job #	2211-47226-	01			Arden Rd		Î
Tel 604-596-0391, Fa:	x 604-584-5050													I
				N/S Stree		Arden Rd	Date:	<b>14</b> / - de d					1	North
_				E/W Stree	et	Lake Trail F	Date	Wednesday 3/30/2022	/					
Pa	ssenge	er Cars		City, Prov	vince	Courtenay,	•	0/00/2022		Lake T	rail Rd		Lake Trail Ro	d
				•		•	Weather:					Г		
							AM:	Cloudy						
							Mid: PM:	Sun / Cloud	lo.			 Arden Rd		
							PIVI.	Suii / Cioud	15			Alueli Ku		
Time		Northbound			Southbound	d		Westbound			Eastbound		15min	1 hour
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
<b>AM</b> 6:00 - 6:15					1			1					0	
- 6:30													0	
- 6:45													0	
- 7:00 7:00 - 7:15	2		1	1			4	11			19	2	0 40	0 40
- 7:30	2		2	<u> </u>			1	14			23	4	46	86
- 7:45	4		2		1		1	16			17	4	45	131
- 8:00 8:00 - 8:15	4	1	4	<b>-</b>	1		1	20 17		2	20 32	7	57 60	188 208
- 8:30	3		2		1		1		1		31	3	58	220
- 8:45	5	4	3	1		1		15			26	10	60	235 229
- 9:00 Midday		1	1			ı	2	19			17	10	51	229
11:00 - 11:15													0	
- 11:30													0	
- 11:45 - 12:00													0	0
12:00 - 12:15													0	0
- 12:30 - 12:45				<b>-</b>									0	0
- 12:45 - 1:00													0	0
PM														
3:00 - 3:15 - 3:30	6 4		3	1	1		3	33	2		36 44	7	90 92	
- 3:45	13	1	3	2	1		4	45			45	7	121	
- 4:00	7		1				5	37	2		27	5	84	387
4:00 - 4:15 - 4:30	8 14		2	1			4	39 49	1 2		44 26	3 4	101 101	398 407
- 4:45	17		1	1	1	1	2	49			30	4	101	388
- 5:00	4		4	1			2	49			36	4	100	404
5:00 - 5:15 - 5:30	7		3		1	1	7	40	1		24 22	12 5	90 91	393 383
- 5:45	8		1				1	32			26	6	74	355
- 6:00	9	1	2	1			2	23			27	5	70	325
PEAK HOUR SU	IMMARY 1.												Hourly <sup>1</sup>	Fraffic
AM peak hour	16	1	9	1	2	0	3	68	1	2	109	23	235	
MD peak hour	0	0	0	0	0	0	0	0	0	0	0	0	0	
PM peak hour	42	1	7	3	1	0	17	170	5	0	142	19	407	7
1. Pook bour volu							<u> </u>							

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHE	EET	Location:										
McElhanney Consultin							Job#	2211-47226-	01		1	Arden Rd		Ī
Tel 604-596-0391, Fa:	ight T			N/S Stree E/W Stree City, Pro	et	Arden Rd Lake Trail F Courtenay,	Date	Wednesday 3/30/2022 Cloudy Sun / Cloud		Lake 1	rail Rd	Arden Rd	Lake Trail R	North d
Time		Northbound	ı		Southbound	· ·		Westbound			Eastbound		15min	1 hour
1	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
AM			U			- U			Ü			<u> </u>		
6:00 - 6:15													0	
- 6:30 - 6:45													0	
- 6:45 - 7:00													0	0
7:00 - 7:15													0	0
- 7:30													0	0
- 7:45													0	0
- 8:00							1			-			1 0	<u>1</u>
8:00 - 8:15 - 8:30													0	1
- 8:45													0	1
- 9:00													0	0
Midday											1	1		
11:00 - 11:15 - 11:30				-									0	
- 11:30 - 11:45		1											0	
- 12:00													0	0
12:00 - 12:15													0	0
- 12:30													0	0
- 12:45 - 1:00													0	0
PM													0	
3:00 - 3:15													0	
- 3:30											İ		0	
- 3:45													0	
- 4:00				<b></b>			1	1				-	2	2
4:00 - 4:15 - 4:30		+		<del>                                     </del>			1					<del>                                     </del>	0	2
- 4:45		+					<del>                                     </del>						0	3
- 5:00													0	1
5:00 - 5:15													0	1
- 5:30													0	0
- 5:45 - 6:00													0	0
PEAK HOUR SU	IMMΔ₽V <sup>1.</sup>				1			1			1	1	U	U
LAK HOOK SU	IVIIVI <i>P</i> AFX I												Hourly	Traffic
AM peak hour	0		0	0	0	0	1	0	0	0	0	0	1	
MD peak hour	0		0	0	0	0	0	0	0	0	0	0		
PM peak hour	0	0	0	0	0	0	2	1	0	0	0	0	3	
Ĺ														

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHI	EET	Location:										
McElhanney Consultin Tel 604-596-0391, Fax							Job #	2211-47226-	01		1	Arden Rd		Ī
		rucks		N/S Stree E/W Stree City, Pro	et	Arden Rd Lake Trail I Courtenay,	Date	Wednesday 3/30/2022 Cloudy Sun / Cloud		Lake T	rail Rd	Arden Rd	Lake Trail R	North d
Time		Northbound	1		Southbound	· ·		Westbound			Eastbound		15min	1 hour
Time	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
AM			13		1	13						19	1	
6:00 - 6:15													0	
- 6:30 - 6:45			1										0	
- 6:45 - 7:00													0	0
7:00 - 7:15								1			1		2	2
- 7:30								1					1	3
- 7:45								2					2	5
- 8:00 8:00 - 8:15		+						1			2		2 2	7 7
- 8:30								1			1		2	8
- 8:45													0	6
- 9:00											1		1	5
Midday			ı		T	Ī		1			1	I		
11:00 - 11:15 - 11:30													0	
- 11:45													0	
- 12:00													0	0
12:00 - 12:15													0	0
- 12:30					ļ								0	0
- 12:45 - 1:00													0	0
PM													0	0
3:00 - 3:15							1	3			2		6	
- 3:30													0	
- 3:45								2			3		5	
- 4:00								1			1		2	13
4:00 - 4:15 - 4:30								1			1		1	8 9
- 4:45								'					0	4
- 5:00								1					1	3
5:00 - 5:15	<u>_</u>										1		1	3 2
- 5:30													0	2
- 5:45 - 6:00							<b> </b>	1					1 0	3
PEAK HOUR SU	MMARY 1.	1	l		I.	1		ı			<u>I</u>	l		
l													Hourly	Traffic
AM peak hour	0		0	0	0	0	0	2	0	0	4	0		
MD peak hour	0		0	0	0	0	0	0	0	0	0	0		
PM peak hour	0	0	0	0	0	0	0	4	0	0	5	0	9	
Ļ														

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COUN	T SHI	EET		Location:													
McElhanney Consulting					Location:				Job #	2211-47226	-01					Arden Rd		1
Tel 604-596-0391, Fax									J00 #	2211-47220	-01					Aldelliku		
,					N/S Stree	et		Arden Rd	Date:									North
					E/W Stre	et		Lake Trail I	_	Wednesday	/							
	Ped	lestriar	า		City, Pro	vinos		Courtonou	Date	3/30/2022			Lake T	rail Rd			Lake Trail F	ld.
					City, FIO	vilice		Courtenay,	Weather:							-		
									AM:	Cloudy								
									Mid:									
									PM:	Sun / Cloud	s					Arden Rd		
Time		Sout	h Leg			Nort	h Leg			Fast	t Leg			Wes	t Leg		15min	1 hour
Time	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Child	AO	Senior	Disabled	Totals	Totals
AM																		
6:00 - 6:15																	0	
- 6:30 - 6:45					1												0	
- 7:00																	0	0
7:00 - 7:15																	0	0
- 7:30 - 7:45										2				1			2	3
- 8:00														1			1	4
8:00 - 8:15																	0	4
- 8:30 - 8:45					<u> </u>												0	2
- 9:00																	0	0
Midday				1		1		1				1						
11:00 - 11:15 - 11:30					<u> </u>												0	
- 11:45																	0	
- 12:00																	0	0
12:00 - 12:15 - 12:30					<u> </u>												0	0
- 12:30 - 12:45																	0	0
- 1:00																	0	0
PM			ı		<b>—</b>	1		1		1	I				1 41			
3:00 - 3:15 - 3:30					1 1										1		2	
- 3:45																	0	
- 4:00		4															0	3
4:00 - 4:15 - 4:30		1											3				1 3	2
- 4:45		1			3												6	10
- 5:00						2											2	12
5:00 - 5:15 - 5:30		1			1		-							1			0 2	11 10
- 5:45																	0	4
- 6:00		4															4	6
[																		Traffic
AM peak hour	0	0									0		0	1	0	0	1	
I W poak flour	0 1	· ·	. 0	. 0	†	. 0	. 0	. 0			. 0	1 0		0	. 0	0		
PEAK HOUR SU	IMMARY 1.		0	0	0	0		0	0	0	0 0	0	0 0 3	1 0 0	0 0	0 0	Hourly	)

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

TRAFFIC	COU	NT SHI	EET	Location:										
McElhanney Consultin							Job #	2211-47226-0	01			Arden Rd		1 I
Tel 604-596-0391, Fax														
				N/S Stree		Arden Rd	Date:							North
				E/W Stree	et	Lake Trail I	_	Wednesday	'			_		
	Bicyc	list		City, Prov	dnco	Courtenay,	Date	3/30/2022		Lake 1	Γrail Rd		Lake Trail R	d
I	•			City, FIO	/IIIC <del>C</del>	Courteriay,	Weather:					,-		
I							AM:	Cloudy						
							Mid:	•						
							PM:	Sun / Cloud	s			Arden Rd		
Time		Northbound	ŀ		Southbound	t		Westbound			Eastbound		15min	1 hour
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	Totals	Totals
AM														
6:00 - 6:15 - 6:30													0	
- 6:45													0	
- 7:00													0	0
7:00 - 7:15 - 7:30			1					1			2		3	5
- 7:45			<u>'</u>					2			'	1	3	8
- 8:00											1		1	9
8:00 - 8:15 - 8:30								1 1			2		1 3	8
- 8:45				1				'			1		2	7
- 9:00													0	6
<b>Midday</b> 11:00 - 11:15		1		-	<u> </u>								0	
- 11:30													0	
- 11:45													0	
- 12:00 12:00 - 12:15				-			1						0	0
- 12:30													0	0
- 12:45													0	0
- 1:00 <b>PM</b>				<del> </del>			1						0	0
3:00 - 3:15	1				2	2		2	2		1		10	
- 3:30								1					1	
- 3:45 - 4:00				2				2			1	1	1 5	17
4:00 - 4:15			1					2	1				4	11
- 4:30								3					3	13 13
- 4:45 - 5:00		1						1 2					1 3	13
5:00 - 5:15								5	2		3		10	17
- 5:30	1							1					2	16
- 5:45 - 6:00							1	1 2			1		1 4	16 17
PEAK HOUR SU	MMARY 1.													<b>-</b> "
AM peak hour	0	0	0	1	0	0	0	2	0	0	4	0	Hourly 7	
MD peak hour	0		0	0	0	0			0	0	0	0	0	
PM peak hour	0		1	2	0	0			1	0		1	13	
[		- hd mk h 6 All \/-h								-				

<sup>1.</sup> Peak hour volume based on peak hour of All Vehicles

## **ATTACHMENT C**

Synchro Reports

	٠	<b>→</b>	•	•	•	•	1	<b>†</b>	-	-	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	11	256	17	37	299	65	4	3	22	42	1	17
Future Volume (Veh/h)	11	256	17	37	299	65	4	3	22	42	1	17
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	0.95	0.95	0.95	0.60	0.60	0.60	0.71	0.71	0.71
Hourly flow rate (vph)	14	324	22	39	315	68	7	5	37	59	1	24
Pedestrians					12			2			6	
Lane Width (m)					3.6			3.6			3.6	
Walking Speed (m/s)					1.2			1.2			1.2	
Percent Blockage					1			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	321			326			816	764	349	848	787	355
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	321			326			816	764	349	848	787	355
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			97			97	98	95	76	100	96
cM capacity (veh/h)	1238			1237			274	319	688	250	308	685
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	360	422	49	84								
Volume Left	14	39	7	59								
Volume Right	22	68	37	24								
cSH	1238	1237	516	306								
Volume to Capacity	0.01	0.03	0.09	0.27								
Queue Length 95th (m)	0.3	0.8	2.5	8.7								
Control Delay (s)	0.4	1.0	12.7	21.2								
Lane LOS	Α	Α	В	С								
Approach Delay (s)	0.4	1.0	12.7	21.2								
Approach LOS			В	С								
Intersection Summary												
Average Delay			3.3									
Intersection Capacity Utiliza	ition		51.8%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

Existing 2022 PM Synchro 10 Report DS Page 1

	۶	*	1	1	ļ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	f)	
Traffic Volume (veh/h)	1	4	5	70	55	1
Future Volume (Veh/h)	1	4	5	70	55	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.63	0.63	0.82	0.82	0.65	0.65
Hourly flow rate (vph)	2	6	6	85	85	2
Pedestrians	4			2	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	191	92	91			
vC1, stage 1 conf vol			<u> </u>			
vC2, stage 2 conf vol						
vCu, unblocked vol	191	92	91			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	1.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	99	100			
cM capacity (veh/h)	792	963	1505			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	8	91	87			
Volume Left	2	6	0			
Volume Right	6	0	2			
cSH	914	1505	1700			
Volume to Capacity	0.01	0.00	0.05			
Queue Length 95th (m)	0.2	0.1	0.0			
Control Delay (s)	9.0	0.5	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	9.0	0.5	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilizati	ion		18.4%	IC	U Level c	of Service
Analysis Period (min)			15			
, analysis i silou (iiiii)			10			

Existing 2022 PM Synchro 10 Report DS Page 2

	۶	<b>→</b>	*	•	+	•	1	†	~	1	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	147	19	19	175	5	42	1	7	3	1	1
Future Volume (Veh/h)	1	147	19	19	175	5	42	1	7	3	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.75	0.75	0.75	0.87	0.87	0.87	0.74	0.74	0.74	0.33	0.33	0.33
Hourly flow rate (vph)	1	196	25	22	201	6	57	1	9	9	3	3
Pedestrians		6						2				
Lane Width (m)		3.6						3.6				
Walking Speed (m/s)		1.2						1.2				
Percent Blockage		1						0				
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	207			223			471	464	210	468	473	210
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	207			223			471	464	210	468	473	210
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			88	100	99	98	99	100
cM capacity (veh/h)	1358			1344			490	488	831	494	482	829
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	222	229	67	15								
Volume Left	1	22	57	9								
Volume Right	25	6	9	3								
cSH	1358	1344	519	534								
Volume to Capacity	0.00	0.02	0.13	0.03								
Queue Length 95th (m)	0.0	0.4	3.5	0.7								
Control Delay (s)	0.0	0.9	13.0	11.9								
Lane LOS	Α	Α	В	В								
Approach Delay (s)	0.0	0.9	13.0	11.9								
Approach LOS			В	В								
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utiliza	ition		35.0%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

Existing 2022 PM Synchro 10 Report DS Page 3

	۶	<b>→</b>	*	•	<b>←</b>	•	1	<b>†</b>	~	-	<b>†</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	7	242	7	17	155	18	12	7	30	43	2	11
Future Volume (Veh/h)	7	242	7	17	155	18	12	7	30	43	2	11
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.76	0.76	0.76	0.74	0.74	0.74	0.77	0.77	0.77	0.74	0.74	0.74
Hourly flow rate (vph)	9	318	9	23	209	24	16	9	39	58	3	15
Pedestrians								2			4	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	213			320			626	602	324	655	609	225
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	213			320			626	602	324	655	609	225
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			98			96	98	95	83	99	98
cM capacity (veh/h)	1359			1238			378	401	715	343	398	812
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	336	256	64	76								
Volume Left	9	23	16	58								
Volume Right	9	24	39	15								
cSH	1359	1238	536	390								
Volume to Capacity	0.01	0.02	0.12	0.19								
Queue Length 95th (m)	0.2	0.5	3.2	5.7								
Control Delay (s)	0.3	0.9	12.6	16.5								
Lane LOS	А	Α	В	С								
Approach Delay (s)	0.3	0.9	12.6	16.5								
Approach LOS	V.0	0.0	В	С								
Intersection Summary												
Average Delay			3.2									
Intersection Capacity Utiliza	ation		32.4%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

Existing 2022 AM Synchro 10 Report DS Page 1

	۶	*	1	<b>†</b>	Ţ	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	7	
Traffic Volume (veh/h)	3	7	2	28	55	1
Future Volume (Veh/h)	3	7	2	28	55	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.83	0.83	0.68	0.68	0.76	0.76
Hourly flow rate (vph)	4	8	3	41	72	1
Pedestrians				4	2	
Lane Width (m)				3.6	3.6	
Walking Speed (m/s)				1.2	1.2	
Percent Blockage				0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	122	76	73			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	122	76	73			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	99	100			
cM capacity (veh/h)	873	984	1533			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	12	44	73			
Volume Left	4	3	0			
Volume Right	8	0	1			
cSH	944	1533	1700			
Volume to Capacity	0.01	0.00	0.04			
Queue Length 95th (m)	0.3	0.0	0.0			
Control Delay (s)	8.9	0.5	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	8.9	0.5	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilizat	tion		14.6%	IC	CU Level c	f Service
Analysis Period (min)			15			
			10			

Existing 2022 AM Synchro 10 Report DS Page 2

	٠	<b>→</b>	*	•	-	•	1	†	~	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	113	23	4	70	1	16	1	9	1	2	1
Future Volume (Veh/h)	2	113	23	4	70	1	16	1	9	1	2	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.91	0.91	0.91	0.85	0.85	0.85	0.72	0.72	0.72	0.75	0.75	0.75
Hourly flow rate (vph)	2	124	25	5	82	1	22	1	13	1	3	1
Pedestrians		2										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	83			149			238	234	136	246	246	84
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	83			149			238	234	136	246	246	84
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			97	100	99	100	100	100
cM capacity (veh/h)	1508			1426			712	665	915	696	655	976
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	151	88	36	5								
Volume Left	2	5	22	1								
Volume Right	25	1	13	1								
cSH	1508	1426	772	710								
Volume to Capacity	0.00	0.00	0.05	0.01								
Queue Length 95th (m)	0.0	0.1	1.2	0.2								
Control Delay (s)	0.1	0.5	9.9	10.1								
Lane LOS	Α	Α	Α	В								
Approach Delay (s)	0.1	0.5	9.9	10.1								
Approach LOS			Α	В								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utiliza	ition		18.6%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

Existing 2022 AM Synchro 10 Report DS Page 3

	٠	<b>→</b>	*	•	<b>←</b>	•	1	†	-	1	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	7	257	7	18	164	19	13	7	32	46	2	12
Future Volume (Veh/h)	7	257	7	18	164	19	13	7	32	46	2	12
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.76	0.76	0.76	0.74	0.74	0.74	0.77	0.77	0.77	0.74	0.74	0.74
Hourly flow rate (vph)	9	338	9	24	222	26	17	9	42	62	3	16
Pedestrians								2			4	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	226			340			663	636	344	694	645	239
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	226			340			663	636	344	694	645	239
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			98			95	98	94	81	99	98
cM capacity (veh/h)	1344			1217			356	383	697	321	379	797
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	356	272	68	81								
Volume Left	9	24	17	62								
Volume Right	9	26	42	16								
cSH	1344	1217	517	366								
Volume to Capacity	0.01	0.02	0.13	0.22								
Queue Length 95th (m)	0.2	0.5	3.6	6.7								
Control Delay (s)	0.3	0.9	13.0	17.6								
Lane LOS	A	A	В	С								
Approach Delay (s)	0.3	0.9	13.0	17.6								
Approach LOS			В	С								
Intersection Summary												
Average Delay			3.4									
Intersection Capacity Utiliza	ation		34.0%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

Background 2025 AM
DS
Synchro 10 Report
Page 1

	٠	*	1	<b>†</b>	Ţ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	1→	
Traffic Volume (veh/h)	3	7	2	30	58	1
Future Volume (Veh/h)	3	7	2	30	58	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.83	0.83	0.68	0.68	0.76	0.76
Hourly flow rate (vph)	4	8	3	44	76	1
Pedestrians	<u>'</u>			4	2	'
Lane Width (m)				3.6	3.6	
Walking Speed (m/s)				1.2	1.2	
Percent Blockage				0	0	
Right turn flare (veh)				U	U	
Median type				None	None	
Median storage veh)				INOHE	INOLIC	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	128	80	77			
vC1, stage 1 conf vol	120	00	11			
vC2, stage 2 conf vol						
vCu, unblocked vol	128	80	77			
The state of the s	6.4	6.2	4.1			
tC, single (s)	0.4	0.2	4.1			
tC, 2 stage (s)	2 5	2.2	2.2			
tF (s)	3.5	3.3				
p0 queue free %	100	99	100			
cM capacity (veh/h)	865	979	1528			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	12	47	77			
Volume Left	4	3	0			
Volume Right	8	0	1			
cSH	938	1528	1700			
Volume to Capacity	0.01	0.00	0.05			
Queue Length 95th (m)	0.3	0.0	0.0			
Control Delay (s)	8.9	0.5	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	8.9	0.5	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utiliza	tion		14.6%	IC	CU Level o	f Service
Analysis Period (min)			15	IC.	JO LOVOI C	. COI VIOG
Analysis Feliou (IIIII)			10			

Background 2025 AM
DS
Synchro 10 Report
Page 2

	۶	<b>→</b>	*	1	<b>—</b>	•	1	<b>†</b>	~	-	<b></b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	120	24	4	74	1	17	1	10	1	2	0
Future Volume (Veh/h)	2	120	24	4	74	1	17	1	10	1	2	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.91	0.91	0.91	0.85	0.85	0.85	0.72	0.72	0.72	0.75	0.75	0.75
Hourly flow rate (vph)	2	132	26	5	87	1	24	1	14	1	3	0
Pedestrians		2										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	88			158			250	247	145	261	260	90
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	88			158			250	247	145	261	260	90
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			97	100	98	100	100	100
cM capacity (veh/h)	1501			1416			699	654	905	680	644	970
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	160	93	39	4								
Volume Left	2	5	24	1								
Volume Right	26	1	14	0								
cSH	1501	1416	760	652								
Volume to Capacity	0.00	0.00	0.05	0.01								
Queue Length 95th (m)	0.0	0.1	1.3	0.1								
Control Delay (s)	0.1	0.4	10.0	10.6								
Lane LOS	Α	Α	Α	В								
Approach Delay (s)	0.1	0.4	10.0	10.6								
Approach LOS			Α	В								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utiliza	ation		19.1%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	12	272	18	39	317	69	4	3	23	45	1	18
Future Volume (Veh/h)	12	272	18	39	317	69	4	3	23	45	1	18
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	0.95	0.95	0.95	0.60	0.60	0.60	0.71	0.71	0.71
Hourly flow rate (vph)	15	344	23	41	334	73	7	5	38	63	1	25
Pedestrians					12			2			6	
Lane Width (m)					3.6			3.6			3.6	
Walking Speed (m/s)					1.2			1.2			1.2	
Percent Blockage					1			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	340			346			866	810	370	896	834	376
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	340			346			866	810	370	896	834	376
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			97			97	98	94	73	100	96
cM capacity (veh/h)	1219			1217			253	299	670	230	288	667
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	382	448	50	89								
Volume Left	15	41	7	63								
Volume Right	23	73	38	25								
cSH	1219	1217	495	282								
Volume to Capacity	0.01	0.03	0.10	0.32								
Queue Length 95th (m)	0.3	0.8	2.7	10.5								
Control Delay (s)	0.4	1.1	13.1	23.5								
Lane LOS	A	A	В	C								
Approach Delay (s)	0.4	1.1	13.1	23.5								
Approach LOS	0.4	1.1	В	C								
Intersection Summary												
Average Delay			3.5									
Intersection Capacity Utiliza	ation		53.9%	IC	CU Level o	of Service			Α			
Analysis Period (min)	auOH		15	ıc	O LGVGI C	71 OCI VICE						
Analysis i enou (min)			IJ									

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	J	*	4	<b>†</b>	1	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	₽	
Traffic Volume (veh/h)	1	4	5	74	58	1
Future Volume (Veh/h)	1	4	5	74	58	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.63	0.63	0.82	0.82	0.65	0.65
Hourly flow rate (vph)	2	6	6	90	89	2
Pedestrians	4			2	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	200	96	95			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	200	96	95			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	99	100			
cM capacity (veh/h)	782	958	1500			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	8	96	91			
Volume Left	2	6	0			
Volume Right	6	0	2			
cSH	907	1500	1700			
Volume to Capacity	0.01	0.00	0.05			
Queue Length 95th (m)	0.2	0.1	0.0			
Control Delay (s)	9.0	0.5	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.0	0.5	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utiliz	zation		18.6%	IC	CU Level c	f Service
Analysis Period (min)			15.070	10	JO LOVOI C	. 55, 1100
Alialysis Fellou (IIIIII)			10			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	156	20	20	186	5	45	1	7	3	1	1
Future Volume (Veh/h)	1	156	20	20	186	5	45	1	7	3	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.75	0.75	0.75	0.87	0.87	0.87	0.74	0.74	0.74	0.33	0.33	0.33
Hourly flow rate (vph)	1	208	27	23	214	6	61	1	9	9	3	3
Pedestrians		6						2				
Lane Width (m)		3.6						3.6				
Walking Speed (m/s)		1.2						1.2				
Percent Blockage		1						0				
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	220			237			499	492	224	496	502	223
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	220			237			499	492	224	496	502	223
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			87	100	99	98	99	100
cM capacity (veh/h)	1343			1328			469	470	817	472	464	815
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	236	243	71	15								
Volume Left	1	23	61	9								
Volume Right	27	6	9	3								
cSH	1343	1328	496	514								
Volume to Capacity	0.00	0.02	0.14	0.03								
Queue Length 95th (m)	0.0	0.4	4.0	0.7								
Control Delay (s)	0.0	0.9	13.5	12.2								
Lane LOS	Α	Α	В	В								
Approach Delay (s)	0.0	0.9	13.5	12.2								
Approach LOS			В	В								
Intersection Summary												
Average Delay			2.4									
Intersection Capacity Utiliza	ition		36.1%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	8	257	7	18	164	22	13	7	32	55	2	16
Future Volume (Veh/h)	8	257	7	18	164	22	13	7	32	55	2	16
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.76	0.76	0.76	0.74	0.74	0.74	0.77	0.77	0.77	0.74	0.74	0.74
Hourly flow rate (vph)	11	338	9	24	222	30	17	9	42	74	3	22
Pedestrians								2			4	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	226			340			675	640	344	700	651	241
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	226			340			675	640	344	700	651	241
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)								0.0	<u> </u>		0.0	0.2
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			98			95	98	94	77	99	97
cM capacity (veh/h)	1344			1217			346	380	697	317	375	795
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total												
	358 11	276 24	68 17	99 <b>74</b>								
Volume Left												
Volume Right	9	30	42	22								
cSH	1344	1217	511	368								
Volume to Capacity	0.01	0.02	0.13	0.27								
Queue Length 95th (m)	0.2	0.5	3.7	8.6								
Control Delay (s)	0.3	0.9	13.1	18.3								
Lane LOS	A	Α	В	C								
Approach Delay (s)	0.3	0.9	13.1	18.3								
Approach LOS			В	С								
Intersection Summary												
Average Delay			3.8									
Intersection Capacity Utiliza	ation		35.5%	IC	CU Level of	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	KA.			ર્ન	ĵ»	
Traffic Volume (veh/h)	8	20	6	30	58	2
Future Volume (Veh/h)	8	20	6	30	58	2
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.83	0.83	0.68	0.68	0.76	0.76
Hourly flow rate (vph)	10	24	9	44	76	3
Pedestrians				4	2	
Lane Width (m)				3.6	3.6	
Walking Speed (m/s)				1.2	1.2	
Percent Blockage				0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)					-	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	142	82	79			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	142	82	79			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	98	99			
cM capacity (veh/h)	847	978	1526			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	34	53	79			
Volume Left	10	9	0			
Volume Right	24	0	3			
cSH	935	1526	1700			
Volume to Capacity	0.04	0.01	0.05			
Queue Length 95th (m)	0.9	0.1	0.0			
Control Delay (s)	9.0	1.3	0.0			
Lane LOS	A	Α				
Approach Delay (s)	9.0	1.3	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utiliza	ation		18.0%	IC	CU Level c	f Service
Analysis Period (min)			15			
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	120	25	5	74	1	19	1	14	1	2	1
Future Volume (Veh/h)	2	120	25	5	74	1	19	1	14	1	2	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.91	0.91	0.91	0.85	0.85	0.85	0.72	0.72	0.72	0.75	0.75	0.75
Hourly flow rate (vph)	2	132	27	6	87	1	26	1	19	1	3	1
Pedestrians		2										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	88			159			254	250	146	268	262	90
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	88			159			254	250	146	268	262	90
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			96	100	98	100	100	100
cM capacity (veh/h)	1501			1414			694	651	904	668	641	970
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	161	94	46	5								
Volume Left	2	6	26	1								
Volume Right	27	1	19	1								
cSH	1501	1414	767	693								
Volume to Capacity	0.00	0.00	0.06	0.01								
Queue Length 95th (m)	0.0	0.1	1.5	0.2								
Control Delay (s)	0.1	0.5	10.0	10.2								
Lane LOS	Α	Α	Α	В								
Approach Delay (s)	0.1	0.5	10.0	10.2								
Approach LOS			А	В								
Intersection Summary												
Average Delay			1.9									
Intersection Capacity Utiliza	ation		19.1%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	16	272	18	39	317	79	4	3	23	51	1	20
Future Volume (Veh/h)	16	272	18	39	317	79	4	3	23	51	1	20
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	0.95	0.95	0.95	0.60	0.60	0.60	0.71	0.71	0.71
Hourly flow rate (vph)	20	344	23	41	334	83	7	5	38	72	1	28
Pedestrians					12			2			6	
Lane Width (m)					3.6			3.6			3.6	
Walking Speed (m/s)					1.2			1.2			1.2	
Percent Blockage					1			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	340			346			884	820	370	912	850	382
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	340			346			884	820	370	912	850	382
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			97			97	98	94	68	100	96
cM capacity (veh/h)	1219			1217			244	294	670	224	281	662
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	387	458	50	101								
Volume Left	20	41	7	72								
Volume Right	23	83	38	28								
cSH	1219	1217	488	275								
Volume to Capacity	0.02	0.03	0.10	0.37								
Queue Length 95th (m)	0.4	0.8	2.7	13.0								
Control Delay (s)	0.6	1.0	13.2	25.5								
Lane LOS	A	A	В	D								
Approach Delay (s)	0.6	1.0	13.2	25.5								
Approach LOS	0.0		В	D								
Intersection Summary												
Average Delay			4.0									
Intersection Capacity Utiliza	ition		52.7%	IC	U Level o	of Service			Α			
Analysis Period (min)	-		15		3.27							

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ર્ન	1>	
Traffic Volume (veh/h)	5	12	19	74	58	6
Future Volume (Veh/h)	5	12	19	74	58	6
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.63	0.63	0.82	0.82	0.65	0.65
Hourly flow rate (vph)	8	19	23	90	89	9
Pedestrians	4			2	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	238	100	102			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	238	100	102			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	98	98			
cM capacity (veh/h)	736	954	1491			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	27	113	98			
Volume Left	8	23	0			
Volume Right	19	0	9			
cSH	877	1491	1700			
Volume to Capacity	0.03	0.02	0.06			
Queue Length 95th (m)	0.8	0.4	0.0			
Control Delay (s)	9.2	1.6	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	9.2	1.6	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			1.8			
Intersection Capacity Utilizati	ion		22.3%	IC	CU Level c	of Service
Intersection Capacity Utilizati Analysis Period (min)	on		22.3% 15	IC	CU Level o	of Service

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	156	22	24	186	5	46	1	9	3	1	1
Future Volume (Veh/h)	1	156	22	24	186	5	46	1	9	3	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.75	0.75	0.75	0.87	0.87	0.87	0.74	0.74	0.74	0.33	0.33	0.33
Hourly flow rate (vph)	1	208	29	28	214	6	62	1	12	9	3	3
Pedestrians		6						2				
Lane Width (m)		3.6						3.6				
Walking Speed (m/s)		1.2						1.2				
Percent Blockage		1						0				
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	220			239			510	502	224	510	514	223
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	220			239			510	502	224	510	514	223
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			87	100	99	98	99	100
cM capacity (veh/h)	1343			1326			460	461	816	459	455	815
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	238	248	75	15								
Volume Left	1	28	62	9								
Volume Right	29	6	12	3								
cSH	1343	1326	494	502								
Volume to Capacity	0.00	0.02	0.15	0.03								
Queue Length 95th (m)	0.0	0.5	4.2	0.7								
Control Delay (s)	0.0	1.0	13.6	12.4								
Lane LOS	Α	A	В	В								
Approach Delay (s)	0.0	1.0	13.6	12.4								
Approach LOS			В	В								
Intersection Summary												
Average Delay			2.6									
Intersection Capacity Utiliza	ation		36.4%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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	٠	<b>→</b>	*	1	<b>←</b>	1	1	†	-	<b>/</b>	<b></b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	9	313	9	22	200	23	16	9	39	56	2	15
Future Volume (Veh/h)	9	313	9	22	200	23	16	9	39	56	2	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.76	0.76	0.76	0.74	0.74	0.74	0.77	0.77	0.77	0.74	0.74	0.74
Hourly flow rate (vph)	12	412	12	30	270	31	21	12	51	76	3	20
Pedestrians								2			4	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	274			414			811	778	420	848	788	290
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	274			414			811	778	420	848	788	290
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			97			92	96	92	69	99	97
cM capacity (veh/h)	1291			1143			279	314	632	242	311	747
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	436	331	84	99								
Volume Left	12	30	21	76								
Volume Right	12	31	51	20								
cSH	1291	1143	433	283								
Volume to Capacity	0.01	0.03	0.19	0.35								
Queue Length 95th (m)	0.2	0.6	5.7	12.1								
Control Delay (s)	0.3	1.0	15.3	24.4								
Lane LOS	А	Α	С	С								
Approach Delay (s)	0.3	1.0	15.3	24.4								
Approach LOS			С	С								
Intersection Summary												
Average Delay			4.4									
Intersection Capacity Utiliza	ation		39.8%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

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	۶	*	1	1	<b></b>	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	7	
Traffic Volume (veh/h)	4	9	2	37	71	1
Future Volume (Veh/h)	4	9	2	37	71	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.83	0.83	0.68	0.68	0.76	0.76
Hourly flow rate (vph)	5	11	3	54	93	1
Pedestrians				4	2	
Lane Width (m)				3.6	3.6	
Walking Speed (m/s)				1.2	1.2	
Percent Blockage				0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				140110	140110	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	156	98	94			
vC1, stage 1 conf vol	100	30	J-T			
vC2, stage 2 conf vol						
vCu, unblocked vol	156	98	94			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	99	100			
cM capacity (veh/h)	835	958	1506			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	16	57	94			
Volume Left	5	3	0			
Volume Right	11	0	1			
cSH	916	1506	1700			
Volume to Capacity	0.02	0.00	0.06			
Queue Length 95th (m)	0.4	0.0	0.0			
Control Delay (s)	9.0	0.4	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	9.0	0.4	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilizatio	n		15.0%	IC	CU Level c	f Service
Analysis Period (min)			15.070	10	. 5 257010	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	146	29	5	90	1	21	1	12	1	2	1
Future Volume (Veh/h)	2	146	29	5	90	1	21	1	12	1	2	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.91	0.91	0.91	0.85	0.85	0.85	0.72	0.72	0.72	0.75	0.75	0.75
Hourly flow rate (vph)	2	160	32	6	106	1	29	1	17	1	3	1
Pedestrians		2										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	107			192			303	299	176	316	314	108
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	107			192			303	299	176	316	314	108
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			95	100	98	100	99	100
cM capacity (veh/h)	1478			1375			644	611	870	623	599	946
Direction, Lane #	EB 1	WB 1	NB 1	SB 1			• • •	• • •				
Volume Total	194	113	47									
				5								
Volume Left	2	6	29	1								
Volume Right	32	1	17	1								
cSH	1478	1375	710	652								
Volume to Capacity	0.00	0.00	0.07	0.01								
Queue Length 95th (m)	0.0	0.1	1.7	0.2								
Control Delay (s)	0.1	0.4	10.4	10.6								
Lane LOS	A	A	В	В								
Approach Delay (s)	0.1	0.4	10.4	10.6								
Approach LOS			В	В								
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utiliza	ation		21.0%	IC	CU Level of	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	15	332	22	48	386	84	5	4	28	55	1	22
Future Volume (Veh/h)	15	332	22	48	386	84	5	4	28	55	1	22
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	0.95	0.95	0.95	0.60	0.60	0.60	0.71	0.71	0.71
Hourly flow rate (vph)	19	420	28	51	406	88	8	7	47	77	1	31
Pedestrians					12			2			6	
Lane Width (m)					3.6			3.6			3.6	
Walking Speed (m/s)					1.2			1.2			1.2	
Percent Blockage					1			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	412			422			1058	988	448	1092	1018	456
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	412			422			1058	988	448	1092	1018	456
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												J
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			96			96	97	92	52	100	95
cM capacity (veh/h)	1147			1141			182	231	606	162	221	601
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	467	545	62	109								
Volume Left	19	51	8	77								
Volume Right	28	88	47	31								
cSH	1147	1141	409	205								
Volume to Capacity	0.02	0.04	0.15	0.53								
Queue Length 95th (m)	0.4	1.1	4.2	22.2								
Control Delay (s)	0.5	1.3	15.4	41.0								
Lane LOS	A	Α	C	F1.0								
Approach Delay (s)	0.5	1.3	15.4	41.0								
Approach LOS	0.0	1.0	C	E								
Intersection Summary												
Average Delay			5.4									
Intersection Capacity Utiliza	ation		62.6%	ıc	CU Level o	of Service			В			
Analysis Period (min)	auOH		15	IC	O LEVEL	JI GEI VICE			D			
Analysis Fellou (IIIII)			13									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N/			र्स	7	
Traffic Volume (veh/h)	1	5	6	90	71	1
Future Volume (Veh/h)	1	5	6	90	71	1
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.63	0.63	0.82	0.82	0.65	0.65
Hourly flow rate (vph)	2	8	7	110	109	2
Pedestrians	4			2	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110	110110	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	242	116	115			
vC1, stage 1 conf vol	- 1-	110	110			
vC2, stage 2 conf vol						
vCu, unblocked vol	242	116	115			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	99	100			
cM capacity (veh/h)	740	934	1475			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	10	117	111			
Volume Left	2	7	0			
Volume Right	8	0	2			
cSH	888	1475	1700			
Volume to Capacity	0.01	0.00	0.07			
Queue Length 95th (m)	0.3	0.1	0.0			
Control Delay (s)	9.1	0.5	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.1	0.5	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilizati	ion		20.3%	IC	CU Level c	f Service
Analysis Period (min)			15			

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 Synchro 10 Report

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	۶	<b>—</b>	*	•	+	4	1	†	<i>&gt;</i>	-	Ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	190	24	24	227	6	55	1	9	4	1	1
Future Volume (Veh/h)	1	190	24	24	227	6	55	1	9	4	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.75	0.75	0.75	0.87	0.87	0.87	0.74	0.74	0.74	0.33	0.33	0.33
Hourly flow rate (vph)	1	253	32	28	261	7	74	1	12	12	3	3
Pedestrians		6						2				
Lane Width (m)		3.6						3.6				
Walking Speed (m/s)		1.2						1.2				
Percent Blockage		1						0				
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	268			287			604	597	271	604	610	270
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	268			287			604	597	271	604	610	270
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			81	100	98	97	99	100
cM capacity (veh/h)	1290			1273			398	407	769	397	401	767
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	286	296	87	18								
Volume Left	1	28	74	12								
Volume Right	32	7	12	3								
cSH	1290	1273	426	432								
Volume to Capacity	0.00	0.02	0.20	0.04								
Queue Length 95th (m)	0.0	0.5	6.1	1.0								
Control Delay (s)	0.0	0.9	15.6	13.7								
Lane LOS	Α	Α	С	В								
Approach Delay (s)	0.0	0.9	15.6	13.7								
Approach LOS			С	В								
Intersection Summary												
Average Delay			2.8									
Intersection Capacity Utiliz	ation		40.4%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

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	٠	<b>→</b>	*	1	<b>←</b>	1	1	†	-	1	<b></b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	10	313	9	22	200	26	16	9	39	65	2	19
Future Volume (Veh/h)	10	313	9	22	200	26	16	9	39	65	2	19
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.76	0.76	0.76	0.74	0.74	0.74	0.77	0.77	0.77	0.74	0.74	0.74
Hourly flow rate (vph)	13	412	12	30	270	35	21	12	51	88	3	26
Pedestrians								2			4	
Lane Width (m)								3.6			3.6	
Walking Speed (m/s)								1.2			1.2	
Percent Blockage								0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	274			414			821	780	420	852	792	292
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	274			414			821	780	420	852	792	292
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			97			92	96	92	63	99	97
cM capacity (veh/h)	1291			1143			272	313	632	241	309	745
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	437	335	84	117								
Volume Left	13	30	21	88								
Volume Right	12	35	51	26								
cSH	1291	1143	428	285								
Volume to Capacity	0.01	0.03	0.20	0.41								
Queue Length 95th (m)	0.2	0.6	5.8	15.3								
Control Delay (s)	0.3	1.0	15.4	26.1								
Lane LOS	Α	Α	С	D								_
Approach Delay (s)	0.3	1.0	15.4	26.1								
Approach LOS			С	D								
Intersection Summary		_	_	_	_	_	_	_	_	_	_	_
Average Delay			5.0									
Intersection Capacity Utiliza	ation		40.5%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	2	146	30	6	90	1	23	1	16	1	2	1
Future Volume (Veh/h)	2	146	30	6	90	1	23	1	16	1	2	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.91	0.91	0.91	0.85	0.85	0.85	0.72	0.72	0.72	0.75	0.75	0.75
Hourly flow rate (vph)	2	160	33	7	106	1	32	1	22	1	3	1
Pedestrians		2										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	107			193			306	302	176	324	318	108
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	107			193			306	302	176	324	318	108
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			99			95	100	97	100	99	100
cM capacity (veh/h)	1478			1374			641	609	869	612	597	946
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	195	114	55	5								
Volume Left	2	7	32	1								
Volume Right	33	1	22	1								
cSH	1478	1374	716	648								
Volume to Capacity	0.00	0.01	0.08	0.01								
Queue Length 95th (m)	0.0	0.1	2.0	0.2								
Control Delay (s)	0.1	0.5	10.4	10.6								
Lane LOS	Α	Α	В	В								
Approach Delay (s)	0.1	0.5	10.4	10.6								
Approach LOS			В	В								
Intersection Summary												
Average Delay			1.9									
Intersection Capacity Utiliza	ation		21.5%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	19	332	22	48	386	94	5	4	28	61	1	24
Future Volume (Veh/h)	19	332	22	48	386	94	5	4	28	61	1	24
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.79	0.79	0.79	0.95	0.95	0.95	0.60	0.60	0.60	0.71	0.71	0.71
Hourly flow rate (vph)	24	420	28	51	406	99	8	7	47	86	1	34
Pedestrians					12			2			6	
Lane Width (m)					3.6			3.6			3.6	
Walking Speed (m/s)					1.2			1.2			1.2	
Percent Blockage					1			0			1	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	412			422			1076	998	448	1108	1034	462
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	412			422			1076	998	448	1108	1034	462
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98			96			95	97	92	45	100	94
cM capacity (veh/h)	1147			1141			176	227	606	157	216	597
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	472	556	62	121								
Volume Left	24	51	8	86								
Volume Right	28	99	47	34								
cSH	1147	1141	403	199								
Volume to Capacity	0.02	0.04	0.15	0.61								
Queue Length 95th (m)	0.5	1.1	4.3	27.6								
Control Delay (s)	0.6	1.2	15.6	47.8								
Lane LOS	Α	Α	С	Е								
Approach Delay (s)	0.6	1.2	15.6	47.8								
Approach LOS			С	Е								
Intersection Summary												
Average Delay			6.4									
Intersection Capacity Utiliza	tion		61.5%	IC	CU Level o	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ર્ન	1>	
Traffic Volume (veh/h)	5	13	20	90	71	6
Future Volume (Veh/h)	5	13	20	90	71	6
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.63	0.63	0.82	0.82	0.65	0.65
Hourly flow rate (vph)	8	21	24	110	109	9
Pedestrians	4			2	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	280	120	122			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	280	120	122			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	98	98			
cM capacity (veh/h)	696	930	1467			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	29	134	118			
Volume Left	8	24	0			
Volume Right	21	0	9			
cSH	851	1467	1700			
Volume to Capacity	0.03	0.02	0.07			
Queue Length 95th (m)	0.8	0.4	0.0			
Control Delay (s)	9.4	1.5	0.0			
Lane LOS	Α	Α				
Approach Delay (s)	9.4	1.5	0.0			
Approach LOS	Α					
Intersection Summary						
Average Delay			1.7			
	ion		23.2%	IC	CU Level o	of Service
Analysis Period (min)			15			
Intersection Capacity Utilizati	ion		23.2%	IC	CU Level o	of Service

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Volume (veh/h)	1	190	26	28	227	6	56	1	11	4	1	1
Future Volume (Veh/h)	1	190	26	28	227	6	56	1	11	4	1	1
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.75	0.75	0.75	0.87	0.87	0.87	0.74	0.74	0.74	0.33	0.33	0.33
Hourly flow rate (vph)	1	253	35	32	261	7	76	1	15	12	3	3
Pedestrians		6						2				
Lane Width (m)		3.6						3.6				
Walking Speed (m/s)		1.2						1.2				
Percent Blockage		1						0				
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	268			290			614	606	272	616	620	270
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	268			290			614	606	272	616	620	270
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			81	100	98	97	99	100
cM capacity (veh/h)	1290			1270			391	401	767	387	394	767
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	289	300	92	18								
Volume Left	1	32	76	12								
Volume Right	35	7	15	3								
cSH	1290	1270	425	423								
Volume to Capacity	0.00	0.03	0.22	0.04								
Queue Length 95th (m)	0.0	0.6	6.5	1.1								
Control Delay (s)	0.0	1.1	15.8	13.9								
Lane LOS	Α	Α	С	В								
Approach Delay (s)	0.0	1.1	15.8	13.9								
Approach LOS			С	В								
Intersection Summary												
Average Delay			2.9									
Intersection Capacity Utiliza	ition		40.8%	IC	U Level	of Service			Α			
Analysis Period (min)			15									

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## **ATTACHMENT D**

Statement of Limitations

Our File: 2211-47226-01

## **Statement of Limitations**

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